



April 14, 2020

State of Tennessee
Dept. of Commerce and Insurance
Division of Fire Prevention
Manufactured Housing Section
500 James Robertson Parkway
Third Floor, Davy Crocket Tower
Nashville, TN 37243-1162

RE: Rolling Acres Cabins

Munfordville, KY

Model: RAC- 10 RAC-Gallant Revised pages per your review

Dear Joel:

Enclosed please find one set of accepted documents for the above-noted manufacturer.

PFS Corporation has reviewed the enclosed documents and to the best of our knowledge has found them to conform to the State of Tennessee Regulations and the codes contained therein.

Should you have any questions, please feel free to contact this office at any time.

Tim Duscho

Sincerely:

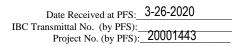
Tim Busche Plan Reviewer

Enclosures: Plans & Calcs

cc: Ammon Miller

Raymond Miller

File





Current PFS Building System Acceptance #:_

ADDITIONAL OR MODIFIED ACCEPTANCE (MODULARS/PANELIZED)

This form is to be used only when the manufacturer is seeking acceptance of an additional model, modified model or model name change which uses a previously accepted building system.

Manufacturer's Name: Rolling Acres Cabins Plants) at which model will be produced 166 John Logsdon Cemetery Rd., Munfordville, KY heck One:	
TECHNICAL DATA TECHNICAL DATA Floor Plan Showing: Braced Wall Method or Shearwalls Building Size (LxW Dimensions) Room Sizes, Light & Ventilation Schedule Exit Requirements Electrical Outlet Spacing & Smoke Detector Location of Labels & Data Plates Use Group, Type Const., Total Sq.Ft. Area Plumbing System Design or Reference No. Plumbing System Design or Reference No. N/A HVAC/Furnace Size/Model No. N/A Thermal Performance Calculations or Reference No. See Attached Rescheck Electrical Load Calculations or Reference No. See Attached Elect. Calc. Service Size and Location 200 amp See electrical Applicable Building Codes 2006 IRC, 2008 NEC W/Amendments	
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Service Size and Location (200 amp See electrical) Applicable Building Codes 2006 IRC, 2008 NEC w/ Amendments	
Applicable Building Codes 2006 IRC, 2008 NEC w/ Amendments	
Submit model to the followingstates: Tennessee	
*Description of Modification:	
Requested by: Ammon Miller Date: 03/16/2020 (designer)	
For PFS Use	
Staff Plan Reviewer Tim Busche Certification #: B5002446-R3 Date: 4-1-2020	
Structural Calculation(s) Reviewed By: P.E. #: Date: Remarks:	
**(1) copy sent to IBC within 15 days of approval.	
VERBAL APPROVAL GIVEN By Whom:	

THIS FORM SHALL BE FILLED OUT COMPLETELY WITH EACH MODEL ACCEPTANCE OR MODIFICATION PRIOR TO SUBMITTAL TO PE

GLENCO INC.

www.glencoinc.com Tel: (570) 837-0577 217 E. Main Street Middleburg, Pennsylvania 17842



12729

60-1283/313

3/16/2020

PAY TO THE ORDER OF

PFS Corporation

s **160.00

One Hundred Sixty and 00/100**

DOLLARS

PFS Corporation 1507 Matt Pass Cottage Grove, WI 53527 ATTN: Debbie Stephenson

MEMO

RAC-Gallant Kentuckian / Rolling Acres Cabins

ADTHORIZED SIGNATURE

"Ol2?29" CO31312835C 3407780601"

GLENCO INC., Middleburg, PA 17842

12729

PFS Corporation

Date Type Reference 3/16/2020 Bill

Original Amt. 160.00 Balance Due 160.00 3/16/2020 Discount

count Payment 160.00

Check Amount

160.00



PFS Corporation

An Employee-Owned Company

TENNESSEE MODULAR PROGRAM DOCUMENT FILING SUBMITTAL FORM

1507 Matt Pass Cottage Grove, WI 5352

Headquarters

Phone: 608.839.1013 Fax: 608.839.1014

Website www.pfscorporation.com

James A. Rothman, PE
President
jrothman@pfscorporation.com

Regional Offices

Northeast Bloomsburg, PA

570.784.8396

Southcentral Plano, TX 972.424.2740

Western Los Angeles, CA 310.559.7287

Midwest Cottage Grove, WI 608.839.1013

Southeast Wake Forest, NC 919.217.7002





Manufacturer's Name	Rolling Acres Cabins							
Mailing Address	166 JOHN LOGSDON CEMETERY RD							
City/State/Zip	MUNFORDVILLE, KY 42765							
Authorized Representativ	/e Ammon Miller							
Phone ²⁷⁰⁻⁵²⁴⁻⁴²⁵²	Fax 570-524-4254	Email_ammonmiller1@ibyfax.com						

Tennessee Assigned Plan Number	DESCRIPTION OF DOCUMENT SUBMITTAL (Actual Unit Dimensions and Occupancy Group)
RAC-10	14' x 48' OCCUPANCY GROUP: 1 & 2 FAMILY

Modular Building Unit Information (MBU): (Include on Cover Page of Drawing)

Code Edition: 2006 IRC	Number of Stories:1
Construction Type: 5B	MBU Height: 11'-11 3/4"
Seismic Site Class:C	Sprinklered:NO
Seismic Site Class:C	Sprinklered:NO

A \$160 document submittal fee (\$150 TN document file fee and \$10 processing fee) must be attached to this form and must be made payable to:

PFS Corporation
PO Box 366
Cottage Grove, WI 53527-0366

Number: 12729 (TN Plan number)

SUBMITTAL NOTES: (Please review before submitting)

1) Provide one PFS Form S and/or Form M, one document filing submittal form and one check for **each** plan, manual or revisions submitted.

(TN Plan number shown on checks)

- 2) All plans must have a "Tennessee Plan Number", i.e. serial numbers <u>are not acceptable.</u>
- 3) All plan submittals shall be a maximum of 11" x 17" format in a single Adobe PDF format file. **No paper copies will be accepted.**
- 4) Manuals (compliance assurance and building submittals) are considered separate submissions and shall be submitted in a single Adobe PDF format file for each manual type.
- 5) If this is a revision, the Plan Number must be the same as the first submission.

Form 107 Revised 1/24/12

Check Number:_

Project Name & Address:

Gallant

2347 Middlesettlements Road, Maryville TN

ATTENTION LOCAL INSPECTION DEPARTMENT IF THIS STRUCTURE IS IN A THERMAL ZONE MORE STRINGENT THAN THAT LISTED ON THESE PLANS. IS SET ON PILINGS, OR IS INSTALLED AT A MOUNTAIN REGION OR COASTAL HIGH HAZARD SITE SUCH THAT WIND OR OTHER DESIGN PARAMETERS ARE INCREASED, THE DESIGN MUST BE DETERMINED TO BE ADEQUATE FOR THE ACTUAL SITE CONDITIONS. ALTERATIONS MAY BE REQUIRED TO BRING THE HOME INTO COMPLIANCE WITH THE MORE STRINGENT CONDITIONS.

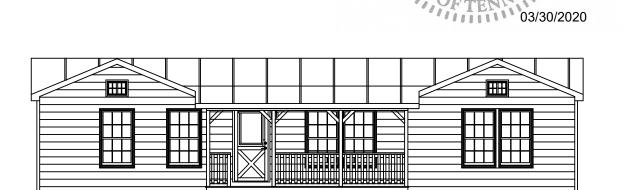
ATTENTION LOCAL INSPECTION DEPARTMENT.
SET-UP INSTRUCTIONS FOR THIS MODULAR UNIT ARE INCLUDED BY ATTACHMENT TO THESE PLANS. ANY PLAN SET WHICH DOES NOT INCLUDE AN ATTACHMENT ENTITLED "INSTALLATION MANUAL" ARE INCOMPLETE. ATTACHED IS OUR INSTALLATION MANUAL APPROVED BY PFS

ATTENTION LOCAL INSPECTION DEPARTMENT

ALL MECHANICAL (DESIGN, MATERIALS, INSTALLATION & INSPECTION) IS TO BE DONE ON-SITE BY OTHERS

<u>ATTENTION LOCAL INSPECTION DEPARTMENT</u>
THE FOLLOWING ITEMS HAVE NOT BEEN COMPLETED BY ROLLING ACRES CABINS, HAVE NOT BEEN INSPECTED BY PFS CORPORATION. CODE COMPLIANCE MUST BE DETERMINED

- EXTERIOR LIGHTS
- PLUMBING CONNECTIONS UNDER THE FLOOR
- CONNECTIONS FROM MODULAR TO FOUNDATION
- COMPLETION OF THE ENERGY CERTIFICATE (SEE ATTACHED ENERGY CODE CALCULATIONS FOR ADDITIONAL ITEMS TO BE INSPECTED ON-SITE)
- FOUNDATION DESIGN AND CONSTRUCTION
- APPLIANCES AND THEIR INSTALLATION PORCHES TO BE COMPLETED ON-SITE
- WINDOW GUARDS PER R613.2



S/N: RAC-GALLANT

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SEAL:

Builder Name: ROLLING ACRES CABINS

SPECIAL USE CONDITIONS, PROVISIONS, LIMITATIONS:

 BUILDING IS DESIGNED PER PROVIDED SETBACKS SHOWN IN CLASSIFICATION SECTION ABOVE. ALL ZONING REQUIREMENTS AND ANY ADDITIONAL FIRE RATINGS REQUIRED ARE THE RESPONSIBILITY OF THE BUILDER.

- FIRE PROTECTION SYSTEM:

 SMOKE ALARMS AS PER STATE CODES

 FLAME SPREAD IS LESS THAN OR EQUAL TO 200
- HEAT LOSS BTU REQUIREMENTS BASED ON THE FOLLOWING:

 FURNACES AND OR HEAT PUMPS TO BE INSTALLED ACCORDING TO ALL
 STATE AND LOCAL CODES W/ MIN.78 AFUE RATING. ALL OTHERS SYSTEMS

- STATE AND LOCAL COURS W MIN./8 AFUE RATING. ALL OTHERS SYSTEMS
 TO MEET ALL STATE AND LOCAL CODES.

 ELECTRIC HEAT (SUPPLED & INSTALLED ON-STE BY OTHERS PER ALL STATE & LOCAL CODES)

 ELEMENT LENGTHS ARE BASED ON 250 WATTS/FT

 HWBB HEAT (SUPPLED & INSTALLED ON-STE BY OTHERS PER ALL STATE & LOCAL CODES)

 ELEMENT LENGTHS ARE BASED ON 550 BTU's/FT @ 180' WATER TEMP.

 AT A FLOW RATE OF 1 GPM USING 3/4" PIPE.

MAXIMUM ELEMENT LENGTH STANDARD): TO STAY WITH	PER LOOP (INDUSTRY IN A 20° TEMPERATURE DROP
PIPE SIZE	MAX. ELEMENT LENGTH
1/2" PIPE	25'-0"
3/4" PIPE	67'-0"
1" PIPE	104'-0"
1 1/4" PIPE	144"-0"

VENTILATION:

- BATH FANS TO PROVIDE A MINIMUM AIR EXCHANGE OF 50 CFM OR
- LARGER DUE TO AREA.
 ALL EXHAUST SYSTEMS TO TERMINATE TO EXTERIOR OF STRUCTURE WITH BACK DRAFT BAFFLE.

GENERAL NOTES

THESE DRAWINGS ARE THE PROPERTY OF ROLLING ACRES CABINS AND SHALL NOT BE REPRODUCED OR COPIED WITHOUT AUTHORIZATION.

OPENINGS FOR EMERGENCY USE (EGRESS WINDOWS & DOORS) TO COMPLY WITH SECTION 310 AND 311 OF THE RESIDENTIAL CODE

ALL STAIR & EGRESS REQUIREMENTS TO COMPLY WITH SECTION R311

 $\underline{\mbox{DISCLAIMER:}}$ These drawings have been prepared and reviewed in accordance with all applicable codes. This drawing set is not intended to be all inclusive, nor does this set detail every code required aspect of this building. The building contractor is responsible for compliance with all applicable codes under the supervision of the local authority having jurisdiction whether detailed in this set or not. Rolling Acres Cabins, its employees and affiliates assume no liability for errors and omissions from this set of drawings as may be permitted by law.



Signature: Staff Plan Reviewer Title:

Date: 4/1/20

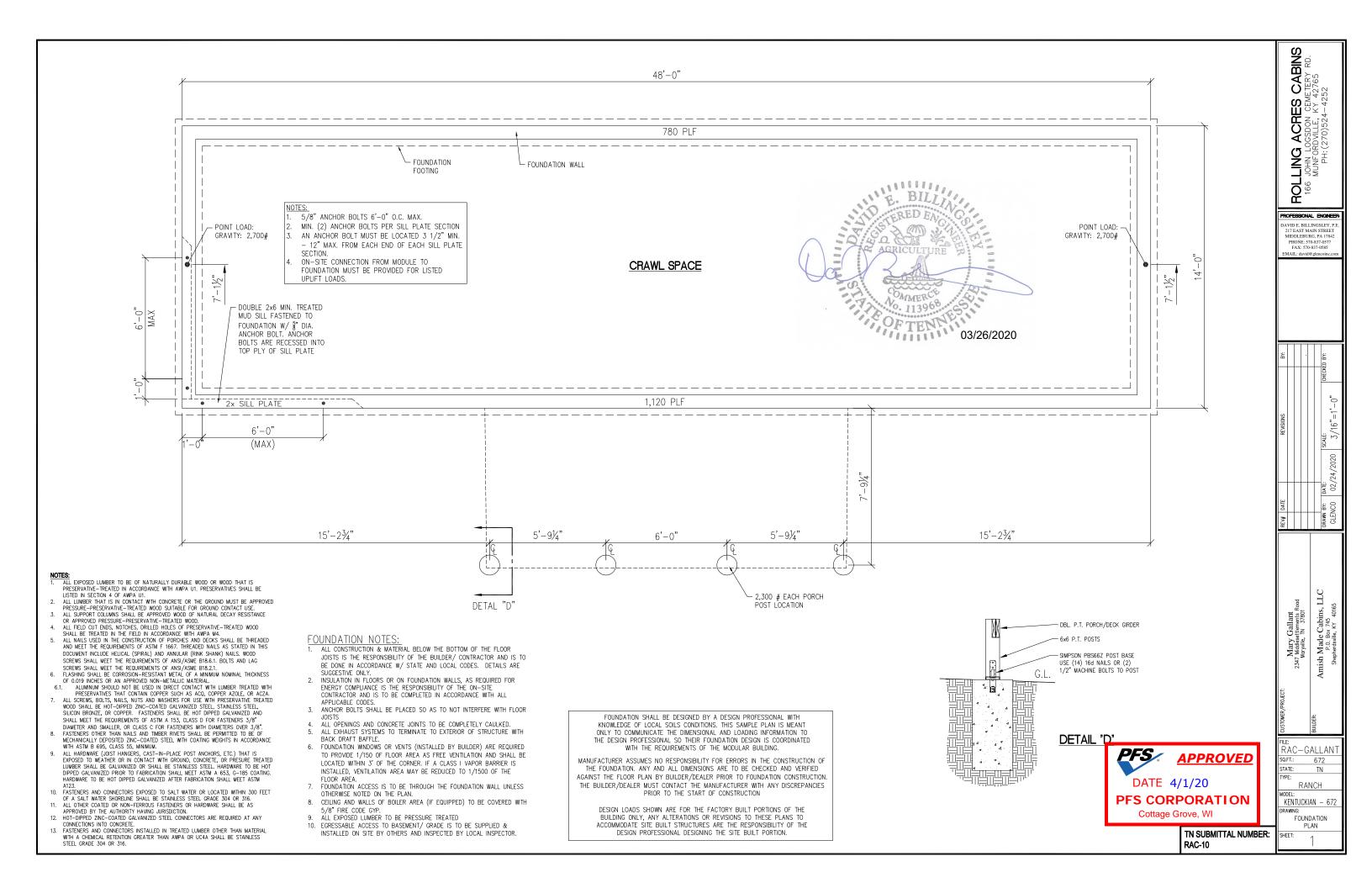
> TN SUBMITTAL NUMBER: RAC-10

9-19.17 TRUSS CERTIFICATION

RAC-GALLAN1 SQ.FT.: 672 STATE: RANCH KENTUCKIAN - 672

COVER SHEET

HEET:



CABINS ETERY RD. 12765 PFS. **APPROVED** FLOOR INSULATION REQUIRED PER ENERGY EFFICIENCY CERTIFICATE NOT PROVIDED BY ROLLING ACRES CABINS IS TO BE PROVIDED AND INSTALLED DATE 4/14/20 ON SITE BY OTHERS PER ATTACHED ENERGY CODE CALCULATIONS ACRES PFS CORPORATION Cottage Grove, WI 48'-0" COMMER 12'-11¾" 11'-10¾" 23'-1½" ROLLING, **Í.** John Munfor_ì PH: ((1) SIMPSON LSTA18 STRAP (1) SIMPSON LSTA18 STRAP (1) SIMPSON LSTA18 STRAP -Eemax WATER HEATER FROM HEADER TO JACK STUD FROM HEADER TO JACK STUD FROM (3) 2x6 SPF #2 MODEL HA018240 04/09/2020 AND JACK STUD TO FLOOR AND JACK STUD TO FLOOR STUDS TO FLOÖR $12^{1}-5\frac{1}{2}$ 5'-0½" 10'-3½" 3'-10½" HEADER H1-1 (2) 2x10 SPF #2 3" RADON VENT SH 2432 HALF GLASS DAVID E. BILLINGSLEY, P 217 EAST MAIN STREET MIDDLEBURG, PA 17842 PHONE: 570-837-0587 FAX: 570-837-0585 EMAIL: david@glencoinc.cc 3/0 W.H. 000 WC2430 1230W W2430 STACK __W/D_ DISP. SB33 DW 2/8 22'-101/2" 4/0 KITCHEN LIVING ROOM (1) SIMPSON LSTA18 STRAP PANEL FROM (3) 2x6 SPF #2 7/16" WOOD STRUCTURAL ∠ LOUVERED DOOR PER M12 14'-0" BATH #1 SHEATHING THIS SIDE OF INTERIOR STACK W/D INSTALLATION WALL UNDER WALL FINISH BEDROOM #1 INSTRUCTIONS 10'-0" BEDROOM #2 51.24 SQ.FT. of ROOM 4.10 LIGHT REQ'D. /2" 79.66 SQ.FT. of ROOM LR/KIT/DIN COMBINED 2.05 VENT REQ'D. 6.37 LIGHT REQ'D. 310.90 SQ.FT. of ROOM 29.44 LIGHT PROV'D. **DINING AREA** 3.19 VENT REQ'D. 24.87 LIGHT REQ'D. 14.72 VENT PROV'D. 15.96 LIGHT PROV'D. 12.44 VENT REQ'D. 7.98 VENT PROV'D. (1) SIMPSON LSTA18 STRAP 54.63 LIGHT PROV'D (3) 2x6 (7) 2x6 (7) FI 22.39 VENT PROV'D. (1) SIMPSON LSTA18 STRAP FROM HEADER TO JACK STUD AND JACK STUD TO FLOOR FROM (4) 2x6 SPF #2 (1) SIMPSON LSTA18 STRAP FROM (2) 2x6 SPF #2 **EGRESS EGRESS** STUDS TO FLOOR SAFETY GLAZED SAFETY GLAZED 3/0 HALF GLASS SH 3662 SH 3662 SH 3662 SH 3662 SH 3662 (1) SIMPSON LSTA18 STRAP (2) 2X6 SPF#2 STUDS FOR -RAILING HALF NEWEL (2) 2X6 SPF#2 STUDS FOR RAILING HALF NEWEL HEADER H1-1 FROM (2) 2x6 SPF #2 (2) 2x10 SPF #2 5½ STUDS TO FLOOR 9'-01/2 10'-3\%" 27'-0" ATTACHMENT ATTACHMENT 5'-41/5" 6'-0" 6'-4½" 10'-9" 7'-0" PORCH ON SITE BY OTHERS Amish Made Cabins, L P.O. Box 745 Chenherdsville, KY 40165 Mary Gallant 7 Middlesettlements R Maryville, TN 37801 7'-0" 7'-0" 6'-0" REVERSE GABLE 6'-0" 6'-0" REVERSE GABLE GL 2418 GL 2418 WINDOW IN DORMER WINDOW IN DORMER 15'-0" 18'-0" 15'-0" PORCH SUPPLIED BY RAC SHIPPED LOOSE AND SITE INSTALLED BY OTHERS DATA PLATE, STATE & 3RD PARTY LABEL LOC'N. THIS HOUSE PLAN MAY BE FLIPPED FROM FRONT TO BACK & OR LEFT TO RIGHT. RAC-GALLANT KITCHEN & BATH ROOM CABINETS MAY BE MOVED, ADDED OR OMITTED. 672 ACTUAL WINDOWS & DOORS MAY BE UPGRADED AS LONG AS THEY ARE EQUAL TO OR FLOOR PLAN SQUARE FOOTAGES BETTER THEN SPECIFIED. RANCH WHIRLPOOL TUB MAY REPLACE THE EXISTING PROVIDING THE LAYOUT WILL NOT MODIFY Total = 672 SQ.FT. of Living Space THE APPROVED PLUMBING DIAGRAM AND THE ELECTRICAL SERVICE CAN SUPPORT THE

ALL WINDOW OPENINGS WHICH ARE 72" ABOVE FINISHED GRADE, WITH THE BOTTOM OF THE

CLEAR OPENING LESS THAN 24" ABOVE FINISHED FLOOR. SHALL BE PROVIDED WITH FIELD

SUPPLIED AND INSTALLED WINDOW GUARDS PER. R613.2

ADDITIONAL LOAD.

TN SUBMITTAL NUMBER:

RAC-10

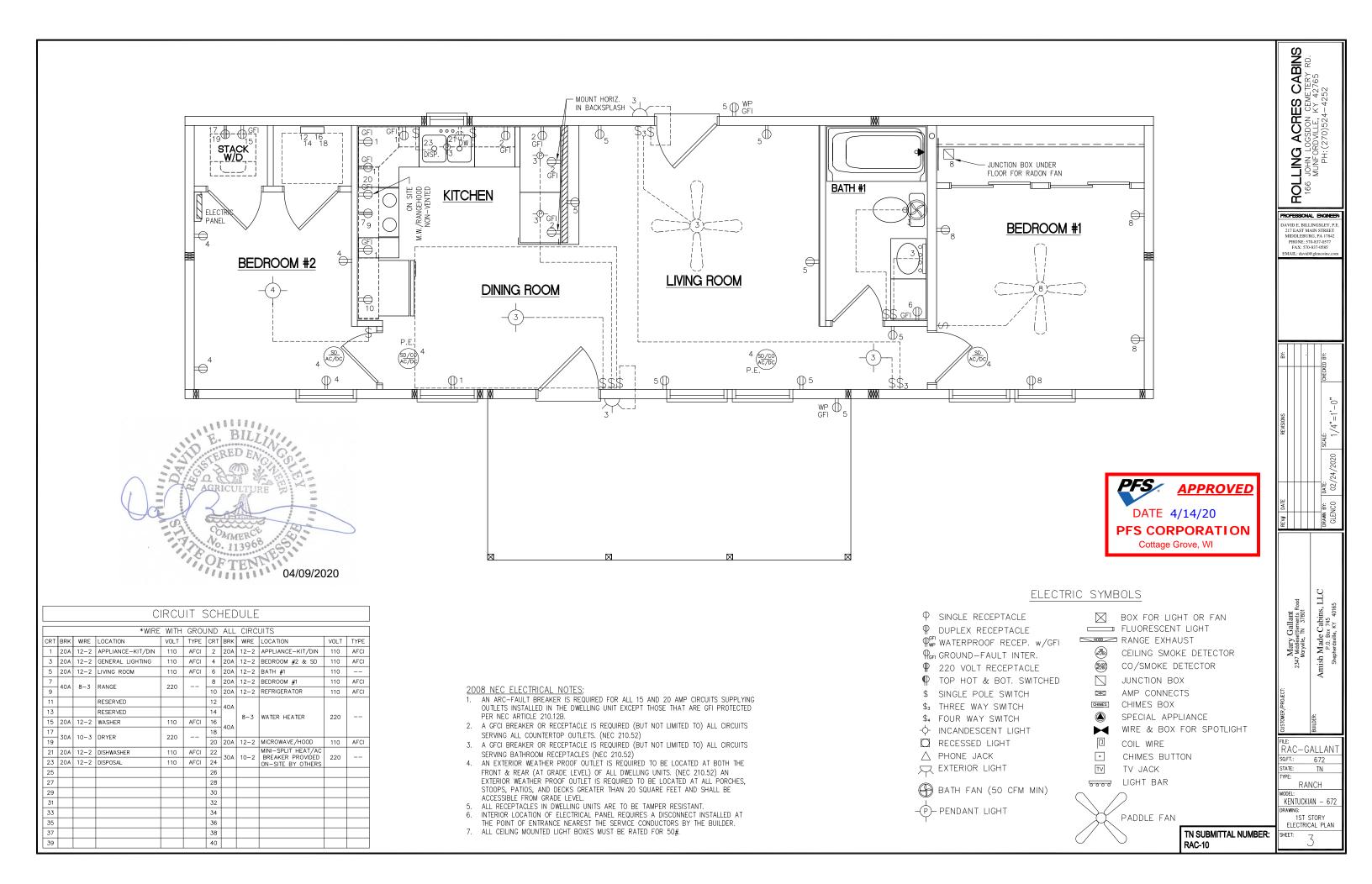
Front Porch = 144 SQ.FT. of Porch

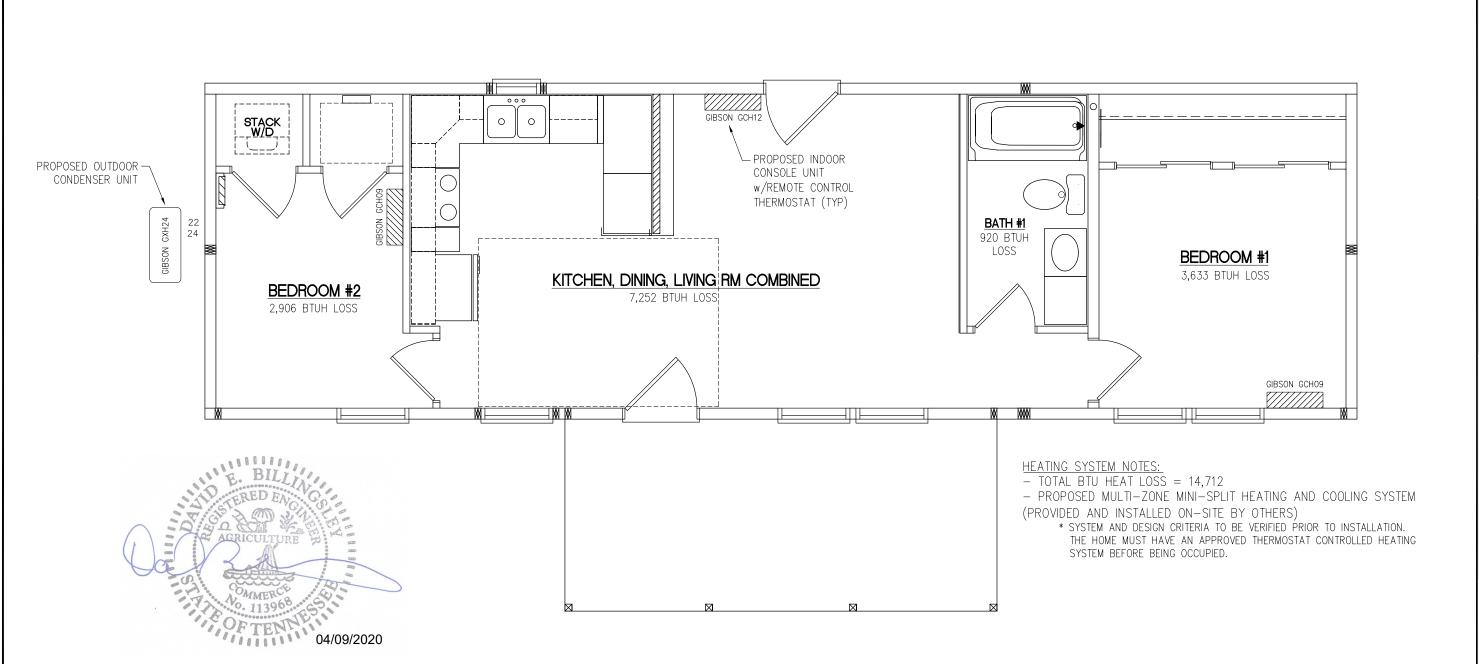
Total = 816 SQ.FT. Under Roof

KENTUCKIAN - 672

1ST STORY

FLOOR PLAN







TN SUBMITTAL NUMBER: RAC-10 ROLLING ACRES CABINS
166 JOHN LOGSDON CEMETERY RD.
MUNFORDVILLE, KY 42765
PH:(270)524-4252

DAVID E. BILLINGSLEY, P. 217 EAST MAIN STREET MIDDLEBURG, PA 17842 PHONE: 570-837-0577 FAX: 570-837-0585 EMAIL: david@glencoinc.co

GLENCO 02/24/2020 1/4"=1'-0"

Mary Gallant
2347 Middlesettlements Rood
Maryville, IN 37801
Amish Made Cabins, LL
P.O. Box 745

FILE:
RAC-GALLANT
SQ.FT.: 672

STATE: TN
TYPE: RANCH

KENTUCKIAN - 672 DRAWING: PROPOSED HEATING PLAN

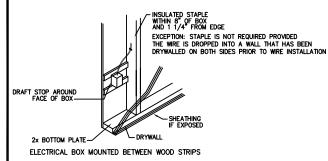
SHEET: 3.1

2008 NEC ELECTRICAL NOTES:

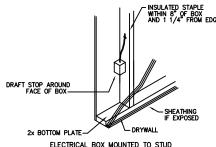
- 1. RECEPTACLES SHALL BE ARRANGED SO THAT AT NO POINT ALONG THE WALL SHALL THE DISTANCE TO A RECEPTACLE BE MORE THAN 6'-0' EXCEPT AT KITCHEN COUNTERTOPS WHERE RECEPTACLES SHALL BE PLACED
- 2. AT LEAST ONE RECEPTACLE SHALL BE INSTALLED IN HALLWAYS OF 10'-0" OR MORE IN LENGTH.
- 3. ANY WALL 2' IN LENGTH OR GREATER REQUIRES A RECEPTACLE (HABITABLE SPACE ONLY).
- 4. ALL COUNTERTOP RECEPTACLES AND SWITCHES TO MEASURE 44" TO BOTTOM OF BOX FROM FINISHED FLOOR.
- 5. WALL SWITCHES TO MEASURE 48" AND RECEPTACLES TO MEASURE 14" TO BOTTOM OF BOX FROM FINISHED
- 7. ALL 125 VOLT, 15 OR 20 AMP RECEPTACLES ARE REQUIRED TO BE LISTED TAMPER RESISTANT (2008 NEC 406.11).
- A COMBINATION TYPE ARC-FAULT BREAKER IS REQUIRED FOR ALL BRANCH CIRCUITS THAT SUPPLY 120V, SINGLE PHASE, 15 AND 20 AMP OUTLETS INSTALLED IN FAMILY ROOMS, DINING ROOMS, LINING ROOMS, PARLORS, LIBRAMES, DENS, BEDROOMS, SUNROOMS, RECREATION ROOMS, CLOSETS, HALLWAYS, AND SIMILAR
- 9. A GFCI BREAKER OR LISTED TAMPER RESISTANT TYPE GFCI RECEPTACLE IS REQUIRED, BUT NOT. LIMITED TO, ALL CIRCUITS SERVING ALL COUNTERTOP OUTLETS (2008 NEC 210.52)
- 10. A GFCI BREAKER OR LISTED TAMPER RESISTANT TYPE GFCI RECEPTACLE IS REQUIRED, BUT NOT LIMITED TO, ALL CIRCUITS SERVING ALL BATHROOM RECEPTACLES (2008 NEC 210.52)
- 11. GFCI PROTECTION IS REQUIRED WHEN OUTLETS ARE WITHIN 6 FEET OF THE OUTSIDE EDGE OF A LAUNDRY, UTILITY, OR WET BAR SINK.'
- 12. A LISTED TAMPER RESISTANT TYPE EXTERIOR WEATHER-RESISTANT/WEATHER-PROOF GFCI OUTLET IS REQUIRED TO BE LOCATED AT BOTH THE FRONT AND REAR OF ALL DWELLING UNITS (2008 NEC 210.52E), OUTLET MUST BE AT GRADE LEVEL. A RECEPTACLE IS ALSO REQUIRED WHEN PORCH/DECK/BALCONIES ARE 20 SQ.FT OR BETTER AND MAY BE USED TO COMPLY WITH 2008 NEC 210.52E WHEN AT GRADE LEVEL AND WITHIN 24"
- SMOKE DETECTORS ARE REQUIRED AT THE FOLLOWING LOCATIONS:
 IS.1. IN EACH BEDROOM
 IS.2. OUTSIDE EACH SEPARATE BEDROOM IN THE IMMEDIATE VICINITY OF THE BEDROOM
 RESERVED.
- 13.3. RESERVED13.4. EACH ADDITIONAL STORY OF THE DWELLING UNIT INCLUDING BASEMENTS, BUT NOT INCLUDING CRAWL SPACES AND UNOCCUPIED ATTICS.
- 14. SMOKE DETECTORS TO BE DIRECTLY CONNECTED TO LIGHTING CIRCUIT WITH NO INTERVENING WALL SWITCH
- 15. SMOKE DETECTORS MOUNTED ON CEILING OR WALL PER CODES & LISTING, NFPA 72, AND ARE TO BE INTERCONNECTED SO THAT IF ONE IS ACTIVATED ALL WILL BE ACTIVATED. SMOKE DETECTORS SHALL BE INSTALLED PER MANUFACTURERS INSTALLATION INSTRUCTIONS.
- CARBON MONOXIDE DETECTORS ARE REQUIRED AT THE FOLLOWING LOCATIONS:

 OUTSIDE OF EACH SEPARATE SLEEPING AREA IN THE IMMEDIATE VICINITY OF THE BEDROOMS IN DWELLING
 UNITS IN WHICH FUEL—FIRED APPLIANCES ARE INSTALLED AND IN DWELLING UNITS THAT HAVE ATTACHED
- 17. CARBON MONOXIDE DETECTOR MOUNTED ON CEILING OR WALL PER CODES & LISTING. WHEN MORE THAN ONE CARBON MONOXIDE DETECTOR IS REQUIRED, THEY MUST BE INTERCONNECTED SO THAT IF ONE IS ACTIVATED ALL OTHERS WILL BE ACTIVATED. CARBON MONOXIDE DETECTORS SHALL BE INSTALLED PER MANUFACTURERS

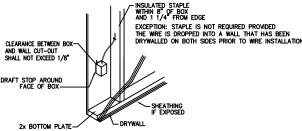
- 18. COMBINATION SMOKE DETECTOR AND CARBON MONOXIDE DETECTORS MAY BE USED.
- 19. BATH FANS MUST BE VENTED TO EXTERIOR
- 20. ADDITIONAL CIRCUITS MAY BE ADDED OR DELETED ON SITE AS FLOOR PLAN OR SALES CONTRACT DICTATE PROVIDED THAT ANY MODIFICATIONS ARE APPROVED BY THE LOCAL CODE INSPECTOR.
- CIRCUITS FOR LAUNDRY, FURNACE AND WATER HEATER INSTALLED IN PANEL BY MANUFACTURER WHEN CRAWL SPACE OPTION IS USED. CIRCUITS ARE INSTALLED BY BUILDER WITH FULL BASEMENT.
- 22. RECESSED LIGHTING FIXTURES WHEN INSTALLED IN THE BUILDING ENVELOPE MUST BE TYPE IC RATED, WITH NO PENETRATIONS BETWEEN INSIDE FIXTURE AND THE CEILING CAVITY, AND GASKETED OR SEALED TO PREVENT LEAKAGE; OR, TYPE IC OR NON-IC RATED INSTALLED INSIDE A SEALED BOX.
- 23. WATER HEATER MAY BE OMITTED BY BUILDER WHEN ALTERNATE SOURCE OF HEAT FOR WATER SUPPLY IS
- 24. STEEL PROTECTORS SHALL BE USED TO PROTECT CABLES LESS THAN 1.25" FROM THE NEAREST EDGE IN ALL
- 25. NON-METALLIC SHEATHED CABLE SHALL BE TYPE NM-B W/GROUND OR MC IN EXPOSED LOCATIONS PER 2011
- 26. CONDUCTORS, ELECTRICAL EQUIPMENT AND LIGHT FIXTURES (LUMINARIES) SHALL BE LABELED OR LISTED BY A MATIONALLY RECOGNIZED TESTING LABORATORY. THE EQUIPMENT SHALL BE SUITABLE FOR LOCATION AND USE IN COMPLIANCE WITH ITS LABEL AND LISTING.
- 27. A 200 AMP PANEL BOX WILL BE USED.
- 28. ELECTRIC DROP AND/OR PANEL BOX LOCATION MAY VARY.
- 29. AN ENERGY EFFICIENCY CERTIFICATE SHALL BE POSTED ON INSIDE OF PANEL BOX.
- WHIRLPOOL TUBS ARE REQUIRED TO BE ON AN INDIVIDUAL CIRCUIT. IF GFI PROTECTION IS PROVIDED FOR THE WHIRLPOOL TUB UNDER THE TUB OR AT THE TUB A ACCESS PANEL SHALL BE READILY ACCESSIBLE
- A WALL SWITCH TO CONTROL LIGHTING OUTLETS FOR INTERIOR STAIRWAYS SHALL BE PROVIDED AT EACH FLOOR LEVEL WHERE STAIRWAYS HAVE SIX OR MORE RISERS.
- 32. LAMP HOLDERS INSTALLED IN WET OR DAMP LOCATIONS SHALL BE OF THE WATER PROOF TYPE.
- 33. WHEN MORE THAN TWO NM CABLES CONTAINING TWO OR MORE CURRENT CARRYING CONDUCTORS ARE INSTALLED WITHOUT MAINTAINING SPACING BETWEEN THE CABLES (OR PLACED IN THE SAME OPENING) SHALL FIRE— OR DRAFT—STOPPED USING THERMAL INSULATION, CAULK OR SEALING FOAM, THE ALLOWABLE PACITY OF EACH CONDUCTOR SHALL BE ADJUSTED IN ACCORDANCE WITH TABLE E3705.3
- 34. ABOVE GROUND CABLES SHALL BE PROTECTED PER THE REQUIREMENTS OF SECTION E3802.
- Panel shall be provided with a main disconnect granted that the panel is installed in a manner that would make the panel a sub-panel.
- 36. A MINIMUM OF 50 PERCENT OF PERMANENTLY INSTALLED LIGHT FIXTURES SHALL CONTAIN ONLY HIGH' EFFICACY LAMPS OR SHALL BE CONTROLLED WITH A OCCUPANCY/VACANCY SENSOR OR AUTOMATED LIGHTING CONTROL



BOXES IN FIRE SEPARATION ASSEMBLIES SHALL NOT BE IN THE SAME STUD CAVITY BETWEEN DWELLING UNITS. BOXES IN SEPARATIONS ARE LISTED FOR THAT USE.



NTRANCE SERVICE HEAD



SGL GANG ELECTRICAL BOX WITH "W" WING BRACKETS UL LISTING 714L

METER AND BASE BY BUILDER GROUND BY BUILDER TYPICAL SERVICE ENTRANCE CLAMP BY BUILDER -

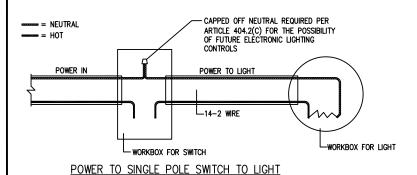
GROUND ROE BY BUILDER

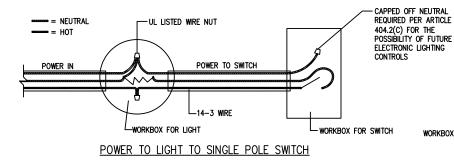
Electrical Load Calculation Form: 3/16/2020 Performed By GLENCO INC Mfg: Rolling Acres Cabins Project: Gallant Model: Ranch Air Conditioning (0 x .65) 0 Central Electric Space Heating Less Than Four Separately Controlled 0 Electric Space Heating Units (0 x .65) Four or More Separately Controlled 0 Electric Space Heating Units (0 x .40) * Use the larger of the air-conditioning or the diversified demand of the heating load. Other Loads Watts or Circuit General Lighting Volt-Amps Ampacity Size 14 x 48 x 3) 2016 15 A 14-2 1st Story 15 A 14-2 2nd Story 0 x 0 x 3) 0 Small Appliances 2 x 1500) 3000 20 A 12-2 11400 50 A 8-3 Range 20 A 12-2 1200 Dishwashe 20 A 12-2 750 Garbage Disposal 12-2 Washe 1500 20 A 5800 30 A 10-3 1920 20 A 12-2 Refrigerator 8-2 4500 40 A Water Heater 40 A Water Heater 4500 8-2 Subtotal: 36586 First 10 KW of other loads @ 100% = 10000 Remainder of other loads @ 40% (26586 x .40) 10634 Heat or A/C from Above: 0 * Total Calculated Load: = 20634 Required Service Size: 20634 ÷ 240 = 85.98 Amps Installed Panel Size 200 Amps

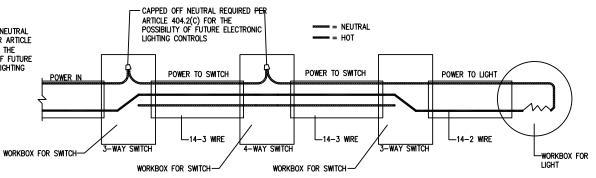




Max panel amps @ 80% =







file name: RAC-Gallant 03-16-2020 JJ

POWER TO 3-WAY SWITCH TO 4-WAY SWITCH TO 3-WAY SWITCH TO LIGHT

TN SUBMITTAL NUMBER: RAC-10

160 Amps

,5020 RAC-GALLAN 672 STATE: **RANCH** KENTUCKIAN - 672 FLECTRICAL NOTES

AND DETAILS

CAE

CRES

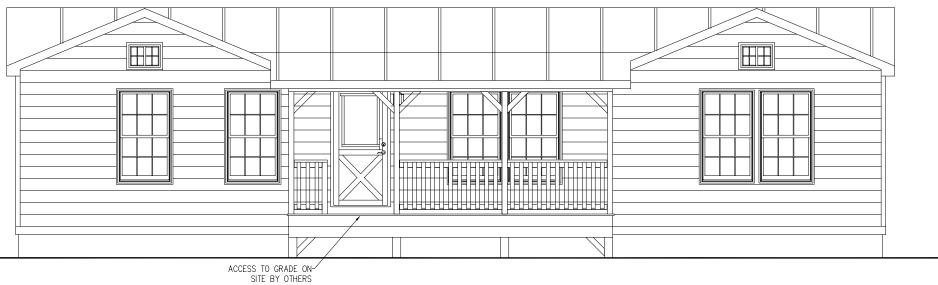
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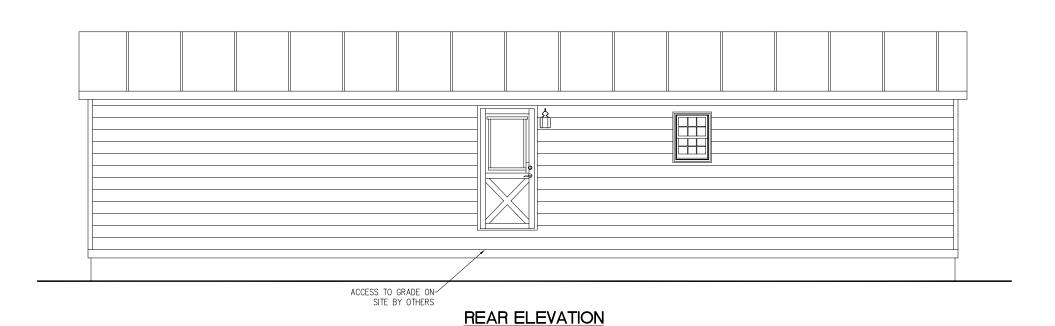
PHONE: 570-837-057 FAX: 570-837-0585

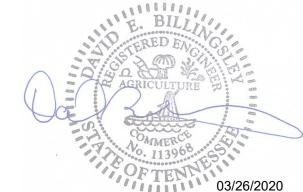
NOTES:

- 1. ELEVATION DRAWINGS ARE APPROXIMATE.
- 2. CORROSION RESISTANT FLASHING AT TOP AND SIDE OF DOORS, WINDOWS, AND AT ROOF PENETRATIONS.
- 3. GUTTERS, LEADERS, AND SPLASH BLOCKS FURNISHED AND INSTALLED BY OTHERS ON SITE.
- 4. ALL ACCESS TO GRADE TO BE ON-SITE BY OTHERS.
- 5. DOWNSPOUT DISCHARGE LOCATIONS AND THE DESIGN OF A DRAINAGE SYSTEM AWAY FROM THE STRUCTURE SHALL BE PROVIDED BY A VA LICENSED DESIGN PROFESSIONAL ON—SITE.



FRONT ELEVATION







TN SUBMITTAL NUMBER: RAC-10 ROLLING ACRES CABINS
166 JOHN LOGSDON CEMETERY RD.
MUNFORDVILE, KY 42765
PH: (270)524-4252

DAVID E. BILLINGSLEY, P.I 217 EAST MAIN STREET MIDDLEBURG, PA 17842 PHONE: 570-837-0577 FAX: 570-837-0585 EMAIL: david@glencoinc.coi

FAX: 5/U-85/-U585

DRAWN BY: DATE. | SCALE. | CHECKED BY | CHEC

COSTONNER/PROJECT:
BUILDER:
BUILDER:
THE:

Amish Made Cabins, LLC P.O. Box 745 Shepherdswile, KY 40165

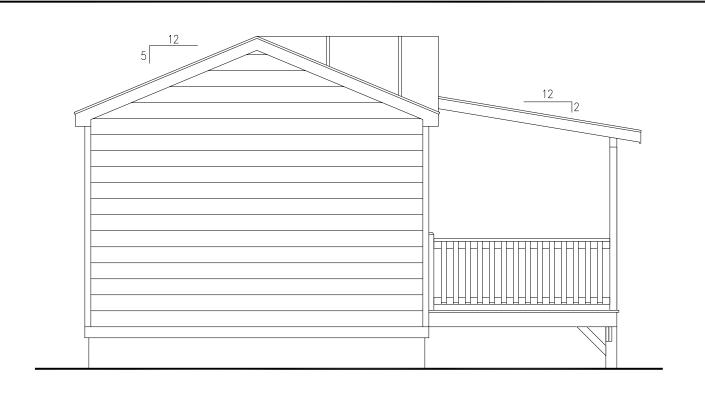
SQ.FT.: 672
STATE: TN

TYPE:

RANCH WODEL: KENTUCKIAN - 672

RENTUCKIAN - 672 PRAWING: FRONT & REAR ELEVATIONS

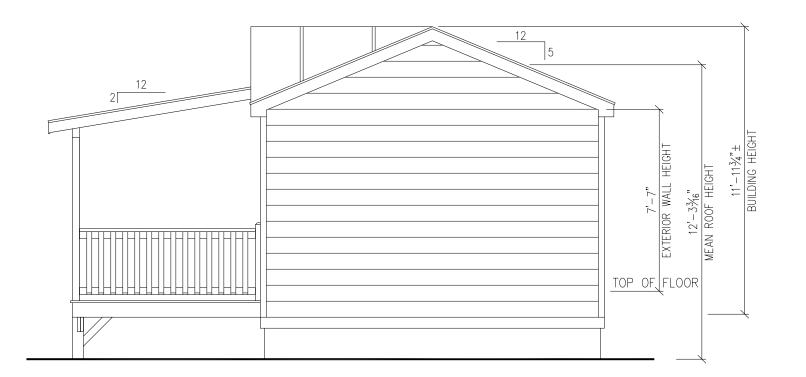
SHEET: 4







LEFT SIDE ELEVATION



RIGHT SIDE ELEVATION

ROLLING ACRES CABINS

ROLLING ACRES CABINS

166 JOHN LOGSDON CEMETERY RD.

166 JOHN LOGSDON CEMETERY RD.

168 JOHN LOGSDON CEMETERY RD.

168 JOHN LOGSDON CEMETERY RD.

168 JOHN LOGSDON CEMETERY RD.

169 JOHN LOGSDON CEMETERY RD.

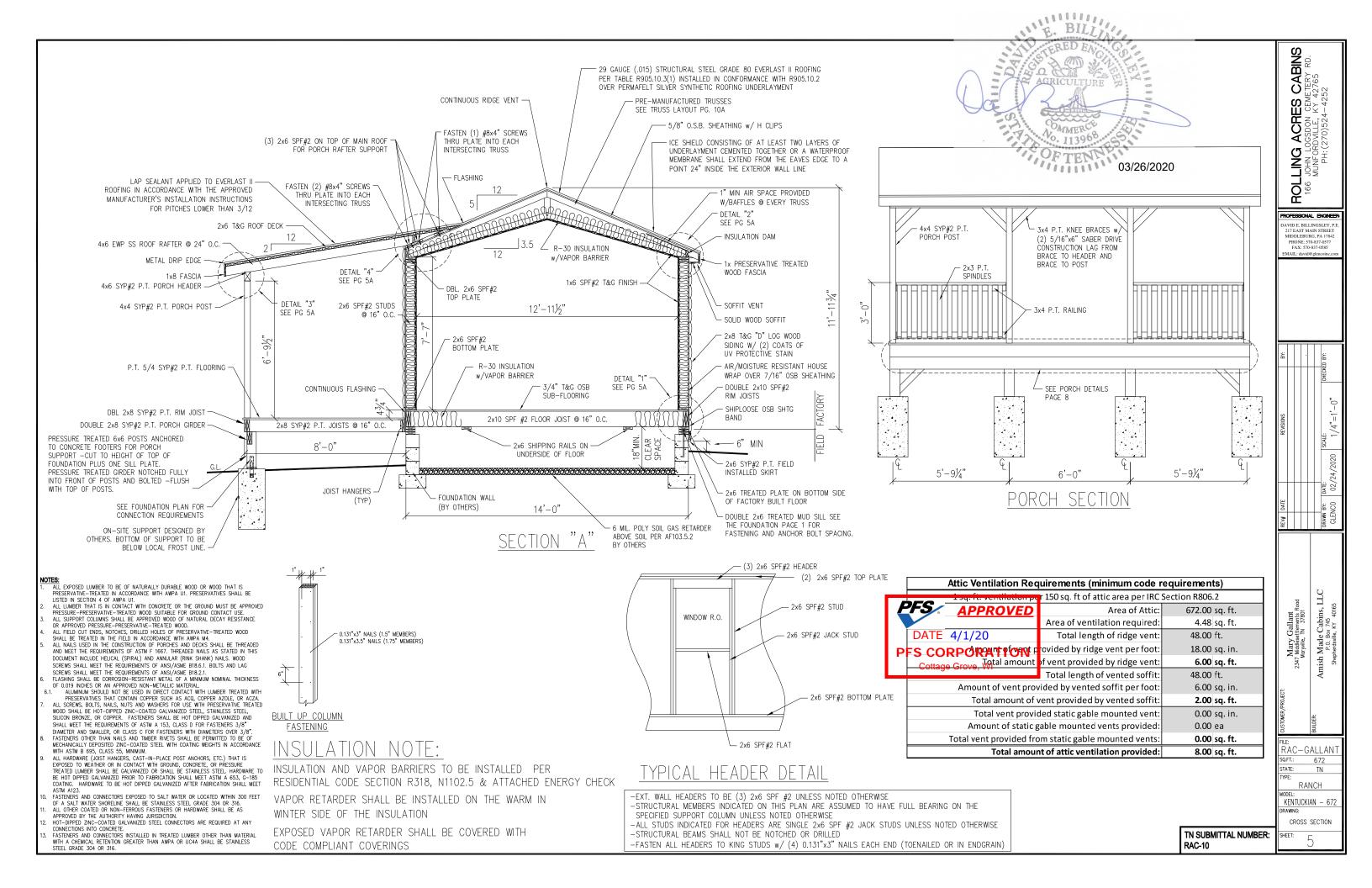
160 JOHN LOGSDON C

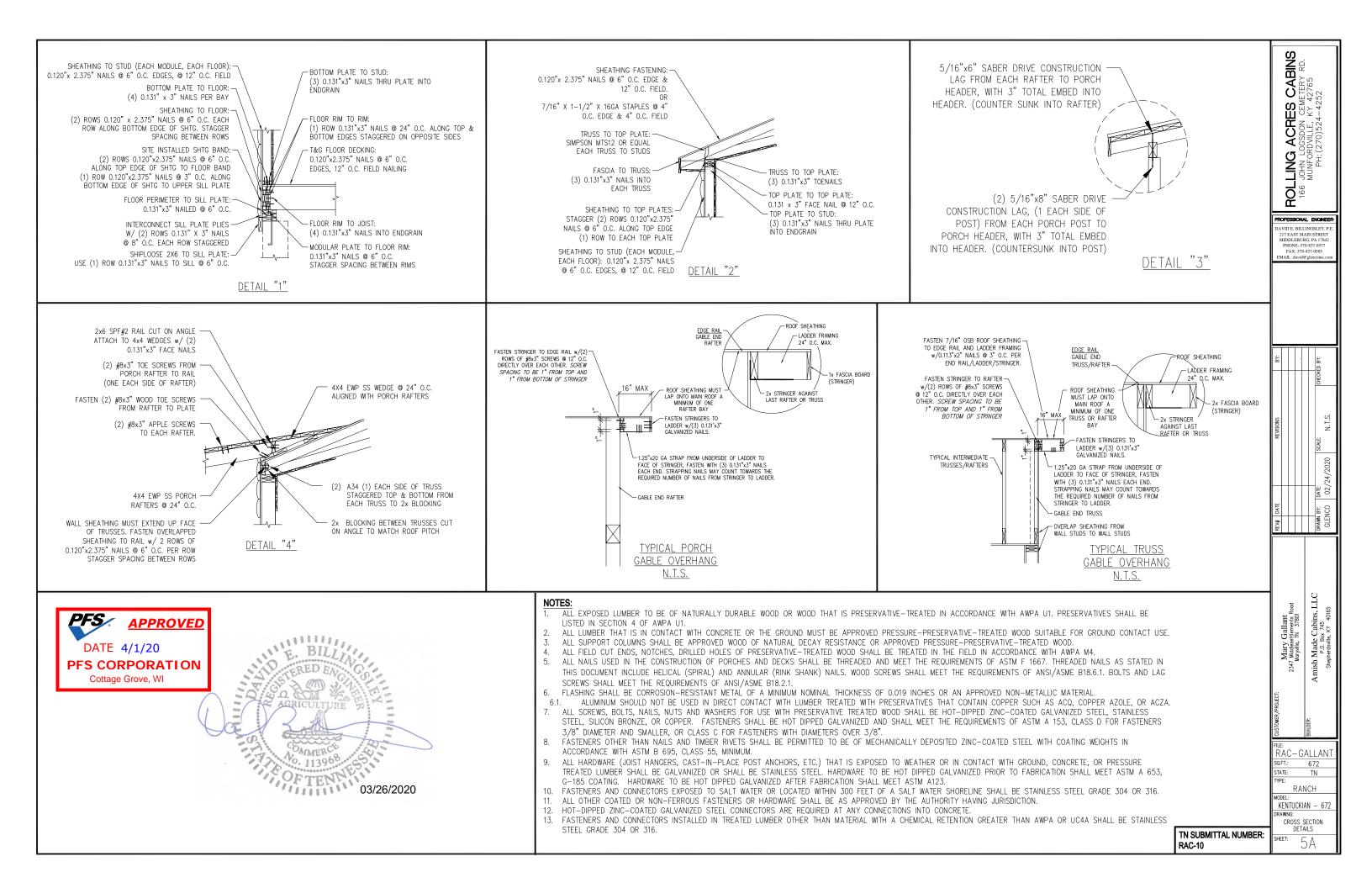
OUSTOMEN/PROACCE:
2347 Middlesettlements Road
Marywille, IN 37801
BUILDER:
Amish Made Cabins, LLC
P.O. Box 745
Shepherdsville, KY 40165

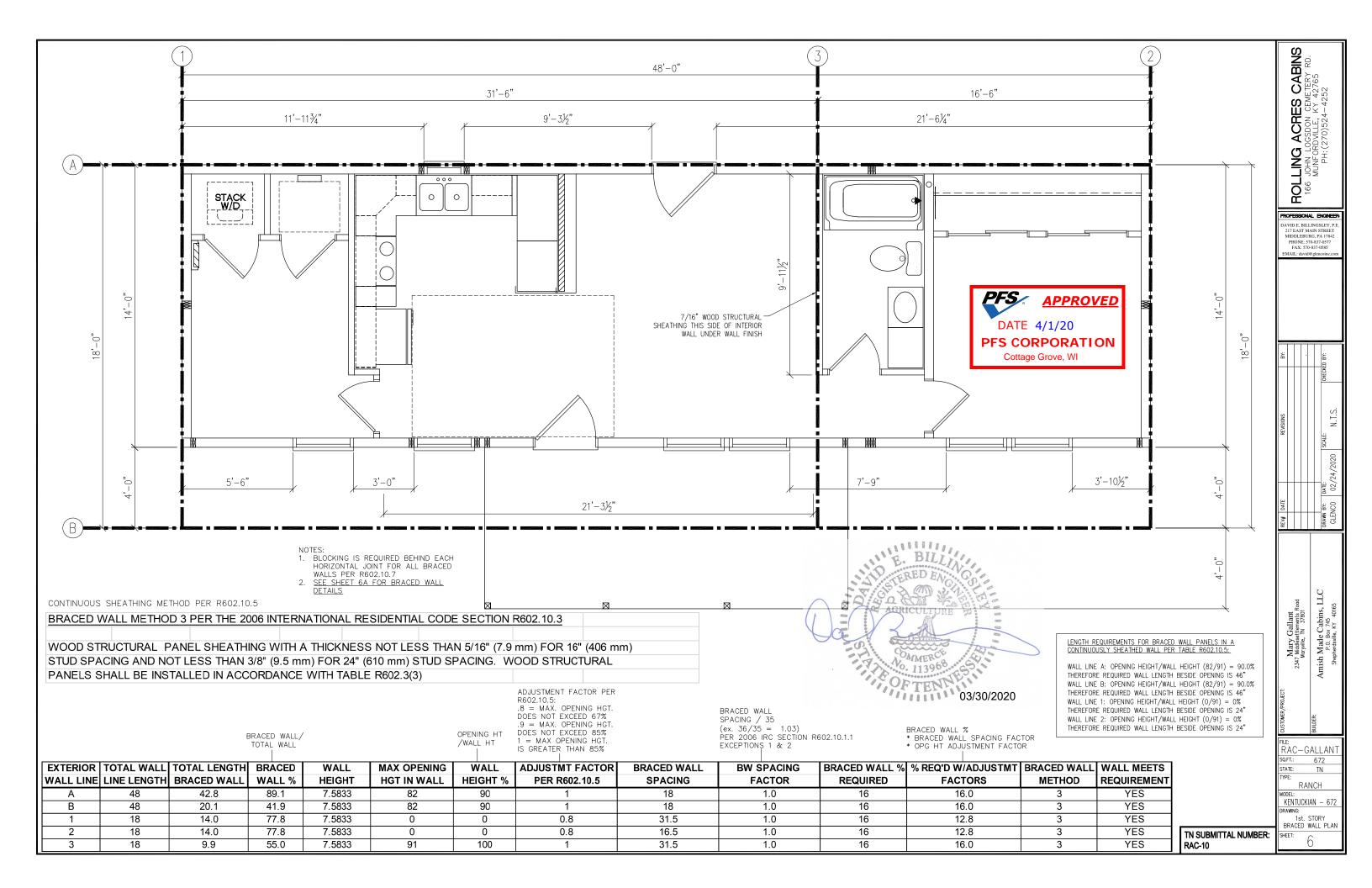
RAC—GALLANT
SQ.FT.: 672
STATE: TN
TYPE: RANCH

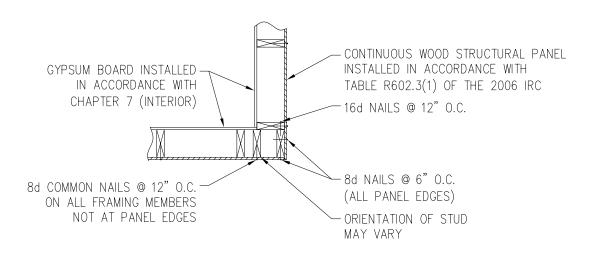
MODEL: KENTUCKIAN – 672 PRAWING: RIGHT & LEFT ELEVATIONS

TN SUBMITTAL NUMBER: SHEET: 4A

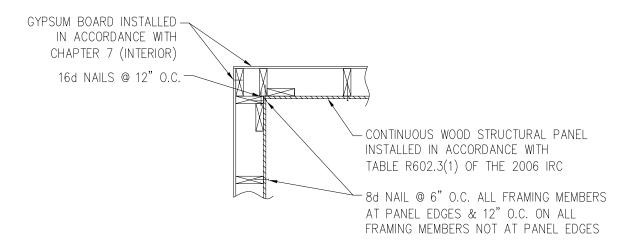








OUTSIDE CORNER DETAIL
FIGURE 602.10.5 OF THE 2006 IRC



INSIDE CORNER DETAIL
FIGURE 602.10.5 OF THE 2006 IRC





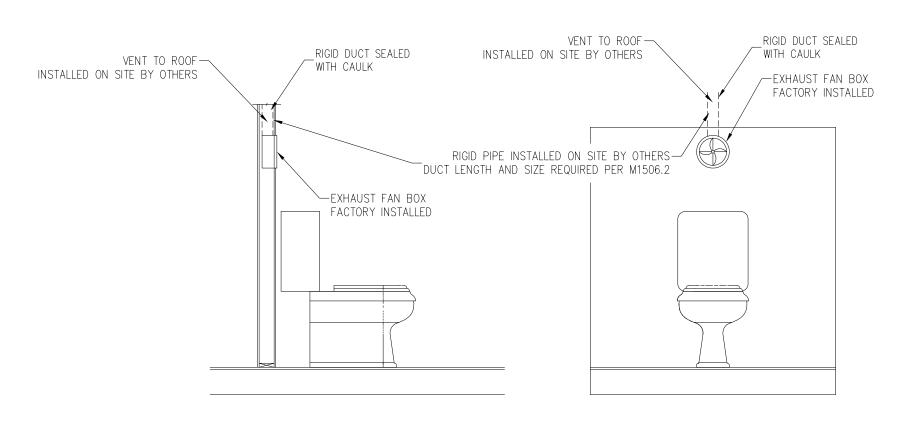
ROLLING ACRES CABINS
166 JOHN LOGSDON CEMETERY RD.
MUNFORDVILLE, KY 42765
PH: (270)524-4252

DAVID E. BILLINGSLEY, P.E. 217 EAST MAIN STREET MIDDLEBURG, PA 17842 PHONE: 570-837-0585 FAX: 570-837-0585 EMAIL: david@glencoinc.com

FILE:
RAC-GALLANT
SQ.FT.: 672
STATE: TN
TYPE:
RANCH

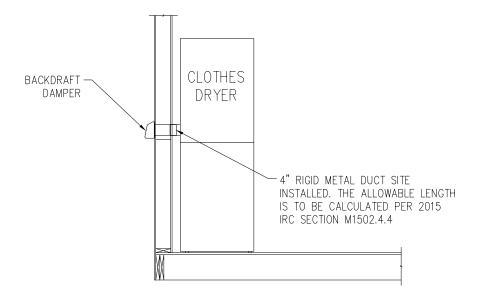
KENTUCKIAN - 672

DRAWING:
BRACED WALL
CONNECTION DETAILS





MECHANICAL VENTILATION DETAIL



DRYER VENT DETAIL

G	4" 90° ELBOW (COUNTS AS 5 LF OF PIPE)
	4" 45° ELBOW (COUNTS AS 2.5 LF OF PIPE)
	BACKDRAFT DAMPER

1. IF

1. IF THE DRYER MAKE & MODEL IS KNOWN, THE MANUFACTURERS INSTALLATION INSTRUCTIONS MUST BE PROVIDED FOR VERIFICATION THAT THE MAXIMUM LENGTH OF THE EXHAUST DUCT INCLUDING TRANSITION DUCT IS IN ACCORDANCE WITH THE INSTALLATION INSTRUCTIONS.

2. DRYER VENT IS TO TERMINATE ON THE OUTSIDE OF THE BUILDING PER THE MANUFACTURERS INSTRUCTIONS.



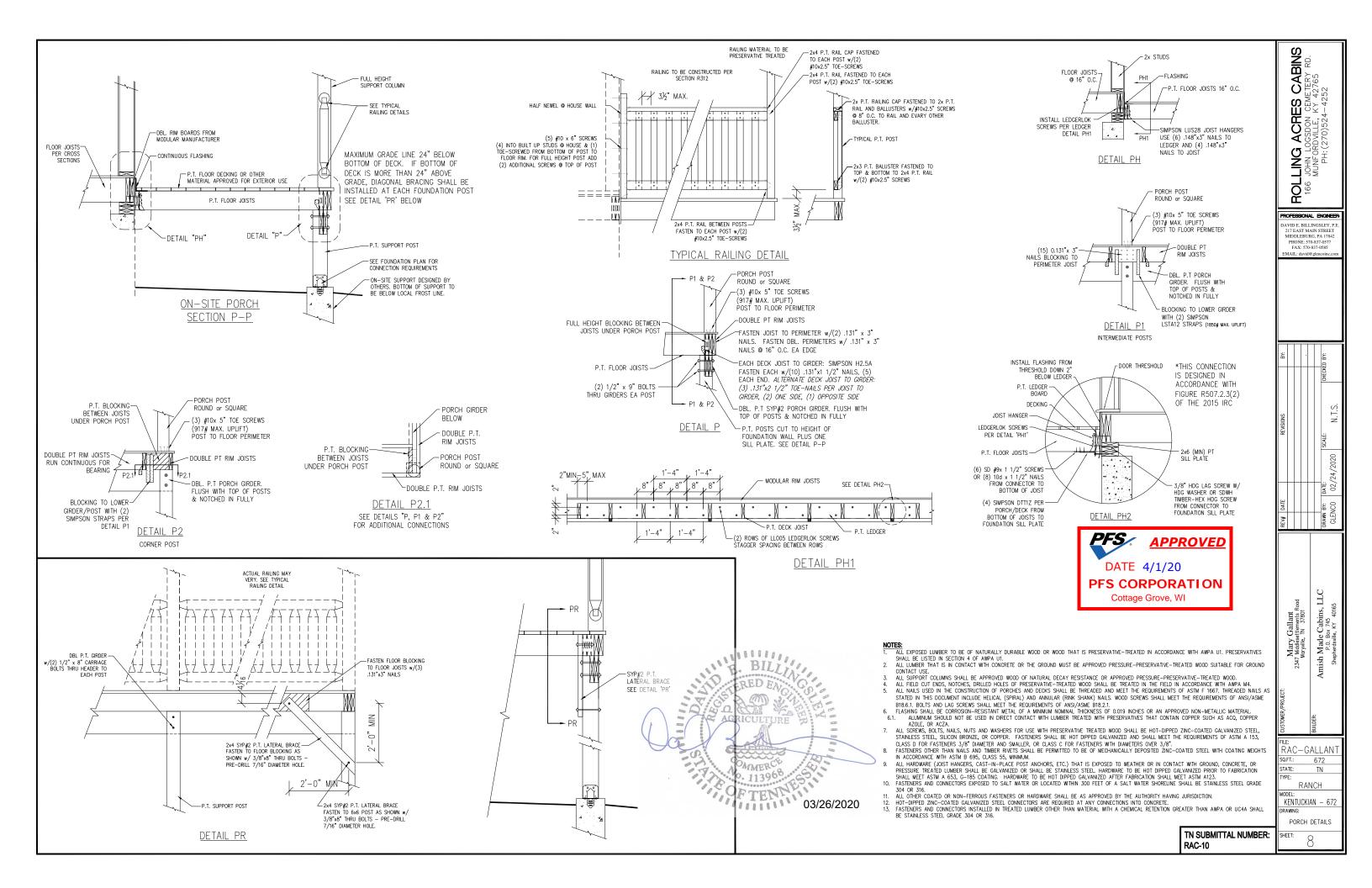
DAVID E. BILLINGSLEY, P.
217 EAST MAIN STREET
MIDDLEBURG, PA 17842
PHONE: 570-837-0535
EMAIL: david@glencoinc.co

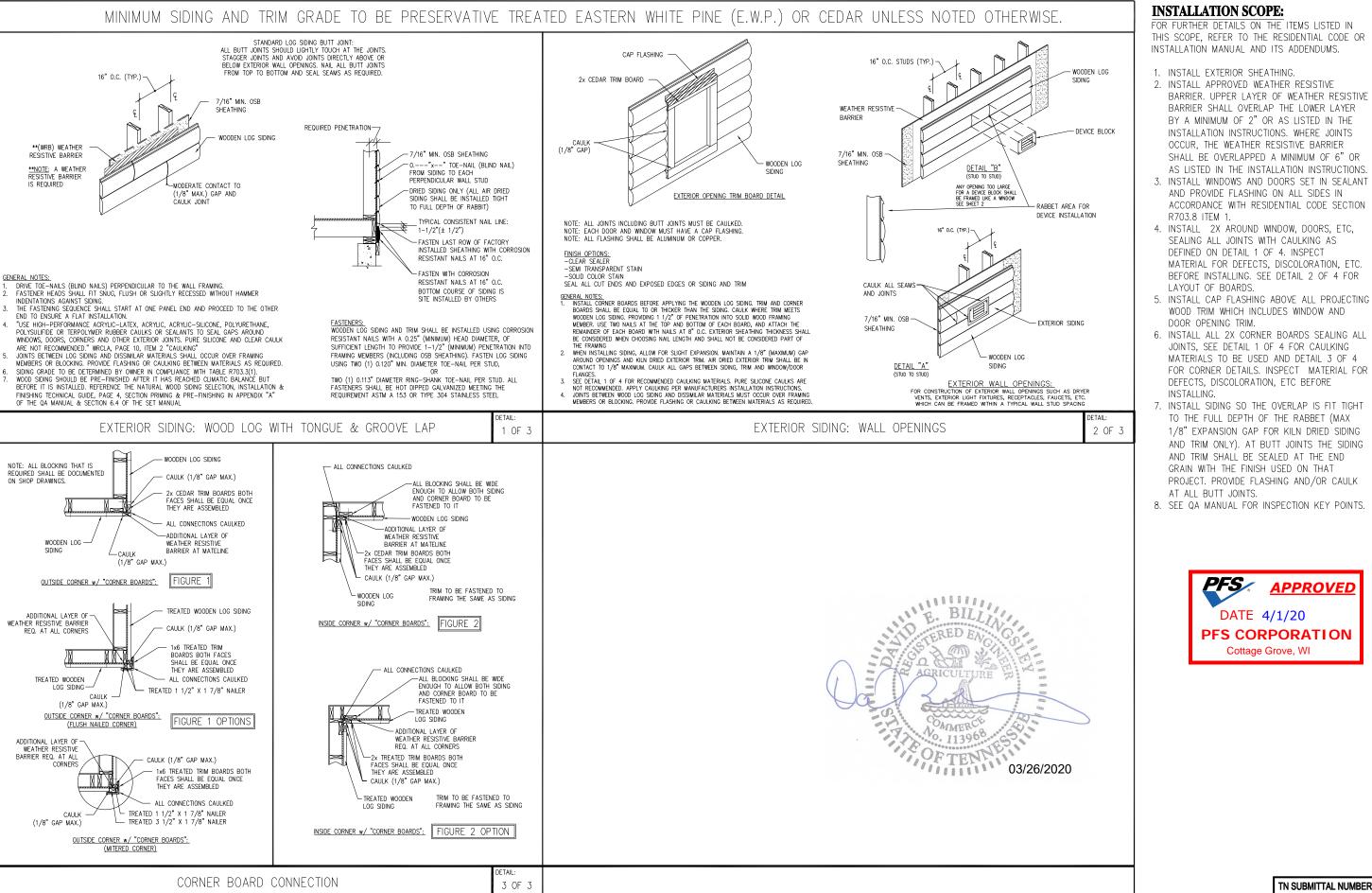
ROLLING ACRES CABINS
166 JOHN LOGSDON CEMETERY RD.
MUNFORDVILLE, KY 42765
PH: (270)524-4252

N FILE: RAC-G.

RAC-GALLANT
SQ.FT.: 672
STATE: TN
TYPE: RANCH

MODEL:
KENTUCKIAN — 672
DRAWING:
MECHANICAL VENT
DETAILS





- BARRIER. UPPER LAYER OF WEATHER RESISTIVE BARRIER SHALL OVERLAP THE LOWER LAYER SHALL BE OVERLAPPED A MINIMUM OF 6" OR
- ACCORDANCE WITH RESIDENTIAL CODE SECTION
- MATERIAL FOR DEFECTS, DISCOLORATION, ETC. BEFORE INSTALLING. SEE DETAIL 2 OF 4 FOR
- MATERIALS TO BE USED AND DETAIL 3 OF 4 FOR CORNER DETAILS. INSPECT MATERIAL FOR
- 1/8" EXPANSION GAP FOR KILN DRIED SIDING AND TRIM ONLY). AT BUTT JOINTS THE SIDING PROJECT. PROVIDE FLASHING AND/OR CAULK
- 8. SEE QA MANUAL FOR INSPECTION KEY POINTS.



CAE ETERY 12765 CRES OLLING 166 JOHN LC 3≥

PHONE: 570-837-057 FAX: 570-837-0585

Amish Made Cabins, I P.O. Box 745 Mary Gallant 7 Middlesettlements R Moryville, TN 37801 RAC-GALLAN 672

RANCH

KENTUCKIAN - 672 WOOD SIDING DETAIL

MINIMUM SIDING AND TRIM GRADE TO BE PRESERVATIVE TREATED EASTERN WHITE PINE (E.W.P.) UNLESS NOTED OTHERWISE.

REMOVE THIS SECTION OPFNING (1)

DETAIL 1 (WATER RESISTIVE BARRIER):

TREATED WOODEN LOG SIDING-

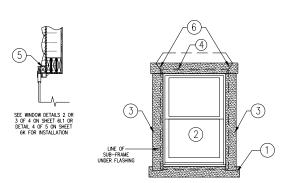
BLOCKING IF REQUIRED FOR WIDER TRIM

KINRO WINDOWS OR EQUAL

INSTALL WATER RESISTIVE BARRIER HORIZONTALLY IN A SHINGLE LIKE FASHION, OVERLAPPING SUCCEEDING COURSE BY A MIN. OF 2

- CUT WATER RESISTIVE BARRIER AT WINDOW R.O. AND WRAP R.O.
- CUT AT A 45° ANGLE TEMP TACK THIS FLAP LIP OUT OF THE WAY

SIDE & TOP SELF-STICK RUBBERIZED FLASHING STARTS IN WINDOW NAILING FLANGE, THEN GOES OUT OVER SUB-FRAME, THEN WRAPS BACK TO WALL SHEATHING PLANE AND EXTENDS OUT UNDER SIDING (THE SILL FLASHING STARTS UNDER THE NAILING FLANGE THEN IT IS INSTALLED THE SAME ON THE SIDE & TOP FLASHING)



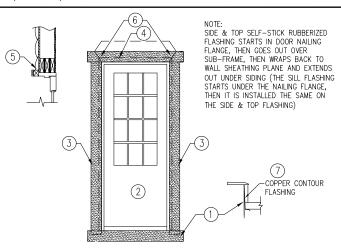
TREATED WOODEN LOG SIDING-

2 OF 4

<u>DETAIL 2</u>
1. INSTALL SELF-STICKING RUBBERIZED FLASHING OVER THE SUB-FRAME AT THE SILL OF WINDOW

INSTALL WINDOW PER MANUFACTURERS INSTALLATION INSTRUCTIONS

- RUN A STRIP OF SELF-STICKING RUBBERIZED FLASHING ON TOP OF THE NAILING FLANGE DOWN THE SIDES OF THE WINDOW AND OVERLAPPING THE SILL FLASHING.
 INSTALL TOP STRIP OF SELF-STICKING RUBBERIZED FLASHING ON TOP OF THE NAILING FLANGE ACROSS THE TOP OF THE WINDOW OVERLAPPING THE SIDE FLASHING
- INSTALL METAL CAP FLASHING OVER THE TOP WINDOW FINISH TRIM BOARD LAP THE TOP STRIP OF WATER RESISTIVE BARRIER DOWN OVER THE TOP STRIP OF RUBBERIZED FLASHING AND THE VERTICAL LEG OF THE METAL CAP FLASHING. TAPE THE 45' SEAMS



<u>DETAIL 3</u>

1. INSTALL SELF-STICKING RUBBERIZED FLASHING OVER THE SUB-FRAME AT THE BOTTOM

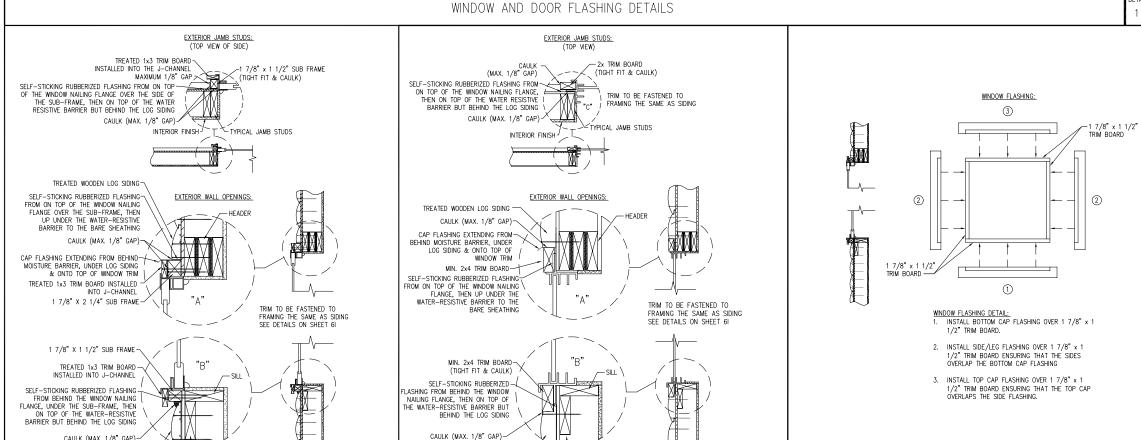
WINDOW FLASHING

INSTALL DOOR PER MANUFACTURERS INSTALLATION INSTRUCTIONS

- RUN A STRIP OF SELF-STICKING RUBBERIZED FLASHING ON TOP OF THE NAILING FLANGE DOWN THE SIDES OF THE DOOR AND OVERLAPPING THE BOTTOM SILL
- INSTALL TOP STRIP OF SELF-STICKING RUBBERIZED FLASHING ON TOP OF THE NAILING FLANGE ACROSS THE TOP OF THE WINDOW OVERLAPPING THE SIDE FLASHING.
 INSTALL METAL CAP FLASHING OVER THE TOP WINDOW FINISH TRIM BOARD
- LAP THE TOP STRIP OF WATER RESISTIVE BARRIER DOWN OVER THE TOP STRIP OF RUBBERIZED FLASHING AND THE VERTICAL LEG OF THE METAL CAP FLASHING. TAPE
- 7. SEE DETAIL FOR ADDITIONAL THRESHOLD FLASHING

1 OF 4

4 OF 4



BLOCKING IF REQUIRED

3 OF 4

PLYGEM WINDOWS OR EQUAL

INSTALLATION SCOPE:

FOR FURTHER DETAILS ON THE ITEMS LISTED IN THIS SCOPE, REFER TO THE RESIDENTIAL CODE OR INSTALLATION MANUAL AND ITS ADDENDUMS.

- 1. INSTALL EXTERIOR SHEATHING.
- 2. INSTALL APPROVED WEATHER RESISTIVE BARRIER. UPPER LAYER OF WEATHER RESISTIVE BARRIER SHALL OVERLAP THE LOWER LAYER BY A MINIMUM OF 2" OR AS LISTED IN THE INSTALLATION INSTRUCTIONS. WHERE JOINTS OCCUR, THE WEATHER RESISTIVE BARRIER SHALL BE OVERLAPPED A MINIMUM OF 6" OR AS LISTED IN THE INSTALLATION INSTRUCTIONS.
- 3. INSTALL WINDOWS AND DOORS SET IN SEALANT AND PROVIDE FLASHING ON ALL SIDES IN ACCORDANCE WITH RESIDENTIAL CODE SECTION R703.8 ITEM 1.
- 4. INSTALL 2X AROUND WINDOW, DOORS, ETC, SEALING ALL JOINTS WITH CAULKING AS DEFINED ON DETAIL 1 OF 4. INSPECT MATERIAL FOR DEFECTS, DISCOLORATION, ETC. BEFORE INSTALLING. SEE DETAIL 2 OF 4 FOR LAYOUT OF BOARDS.
- 5. INSTALL CAP FLASHING ABOVE ALL PROJECTING WOOD TRIM WHICH INCLUDES WINDOW AND DOOR OPENING TRIM.
- 6. INSTALL ALL 2X CORNER BOARDS SEALING ALL JOINTS, SEE DETAIL 1 OF 4 FOR CAULKING MATERIALS TO BE USED AND DETAIL 3 OF 4 FOR CORNER DETAILS. INSPECT MATERIAL FOR DEFECTS, DISCOLORATION, ETC BEFORE INSTALLING.
- 7. INSTALL SIDING SO THE OVERLAP IS FIT TIGHT TO THE FULL DEPTH OF THE RABBET (MAX 1/8" EXPANSION GAP FOR KILN DRIED SIDING AND TRIM ONLY). AT BUTT JOINTS THE SIDING AND TRIM SHALL BE SEALED AT THE END GRAIN WITH THE FINISH USED ON THAT PROJECT. PROVIDE FLASHING AND/OR CAULK AT ALL BUTT JOINTS.
- 8. SEE QA MANUAL FOR INSPECTION KEY POINTS.





TN SUBMITTAL NUMBER: RAC-10

RANCH KENTUCKIAN - 672

OLLING 166 JOHN LC 3≥ PHONE: 570-837-057 FAX: 570-837-0585

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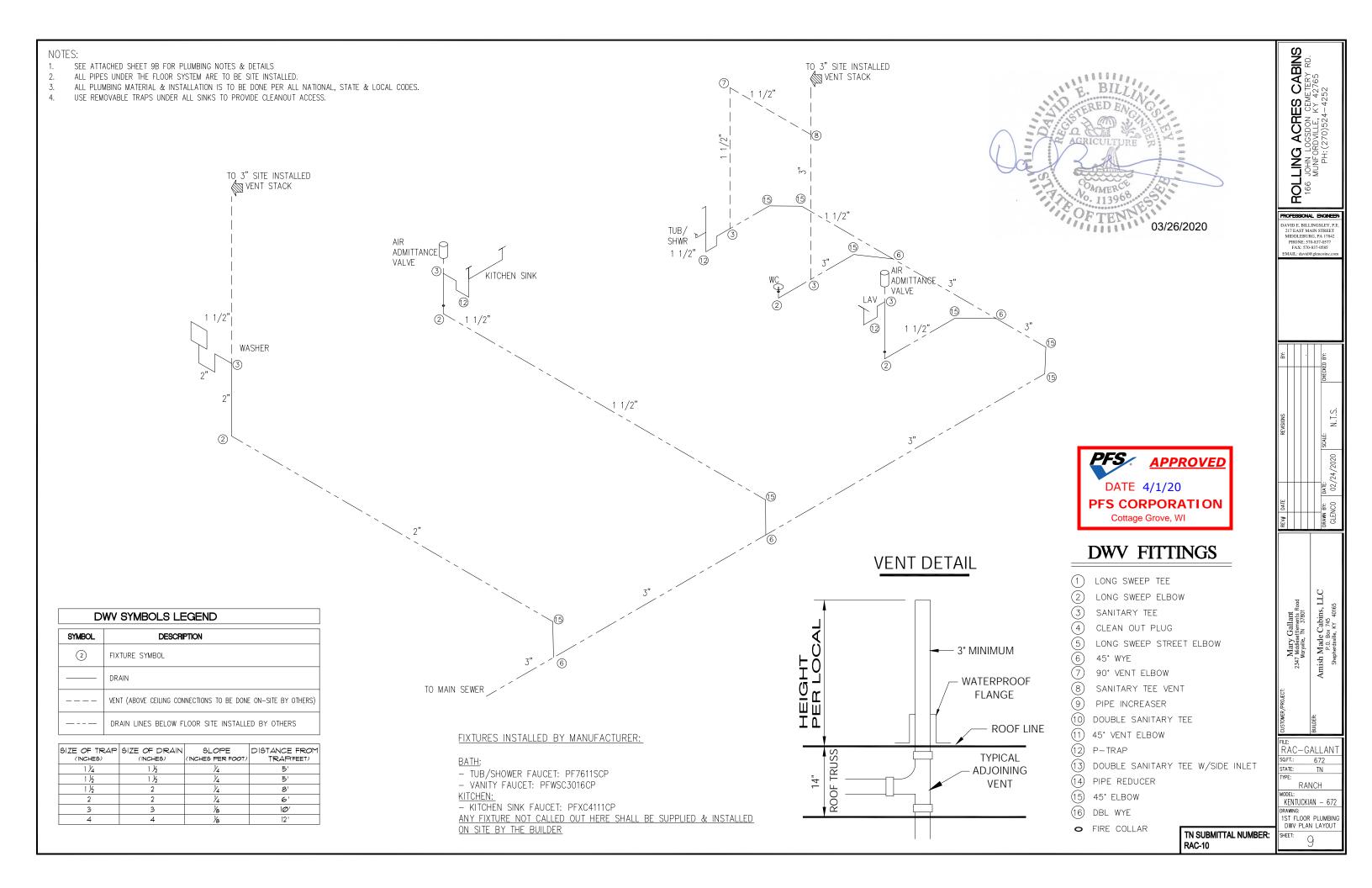
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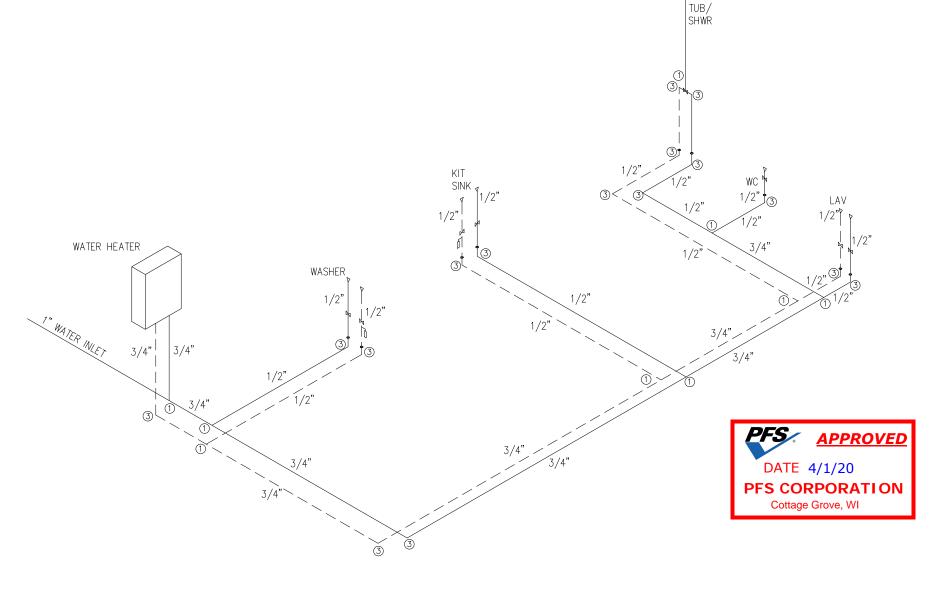
Amish Made Cabins, I. P.O. Box 745
Sheoherdsville, KY 40165 Mary Gallant 7 Middlesettlements R Maryville, TN 37801

RAC-GALLAN 672

WOOD SIDING DETAIL







NOTES:

- 1. SEE ATTACHED SHEET 9B FOR PLUMBING NOTES & DETAILS
- 2. ALL PLUMBING MATERIAL & INSTALLATION IS TO BE DONE PER ALL NATIONAL, STATE & LOCAL CODES.
- 3. A TEMPERATURE-LIMITING DEVICE CONFORMING TO ASSE 1070 IS REQUIRED AT WHIRLPOOL TUBS AND BATH TUBS TO LIMIT WATER TEMPERATURE TO 120°F.
- 4. MAX DEVELOPED LENGTH = 40 FT
- 5. WATER PRESSURE 30-39 PSI MIN.
- 6. SHUT OFF VALVES INSTALLED AT EACH FIXTURE.
- 7. ALL FIRST STORY PLUMBING TO BE STUBBED THRU FLOOR ONLY FOR SITE CONNECTION.

SUPPLY FITTINGS

1 TEE



3 ELL

• FIRE COLLAR

DBL TEE

SHUT OFF VALVE

WATER HAMMER ARRESTOR

SUPPLY SYMBOLS LEGEND

SYMBOL	DESCRIPTION
2	FIXTURE SYMBOL
	COLD
	нот

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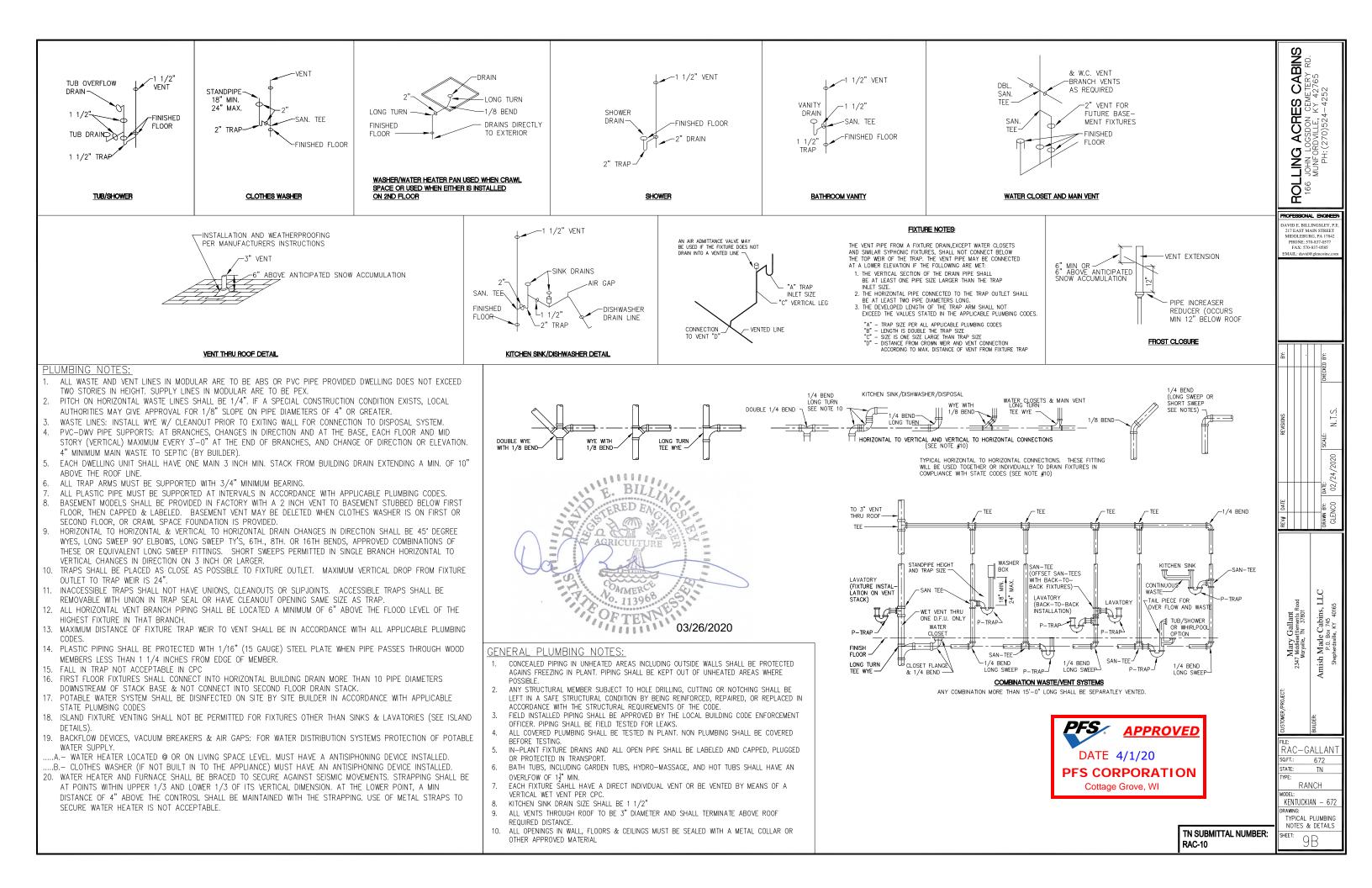
PROFESSIONAL ENGINES

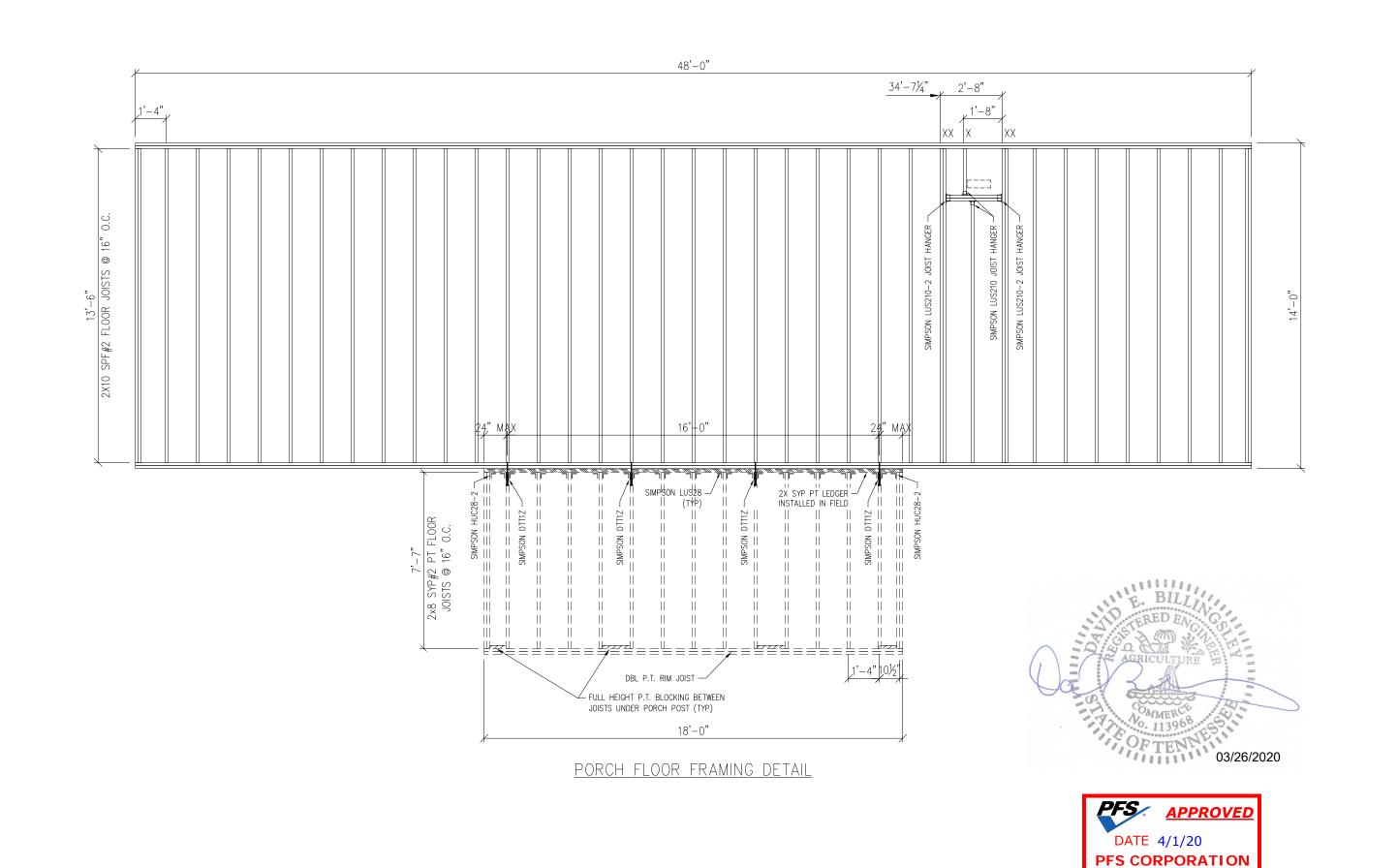
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MELY FIVOREST.	MEV# DAIL		REVISIONS		
Mary Gallant					
2347 Middlesettlements Road					
Maryville, TN 37801					
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Allinsh Made Cabins, LLC	DRAWN BY: DATE:	DATE:	SCALE: CHE	CHECKED BY:	
F.U. Box /45 Shepherdsville, KY 40165	GLENCO	GLENCO 02/24/2020	N.T.S.		

RAC-GALLANT

RANCH
MODEL:
KENTUCKIAN - 672
DRAWNG:
1ST FLOOR PLUMBING
SUPPLY PLAN LAYOUT





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RAC-GALLANT 672 TN

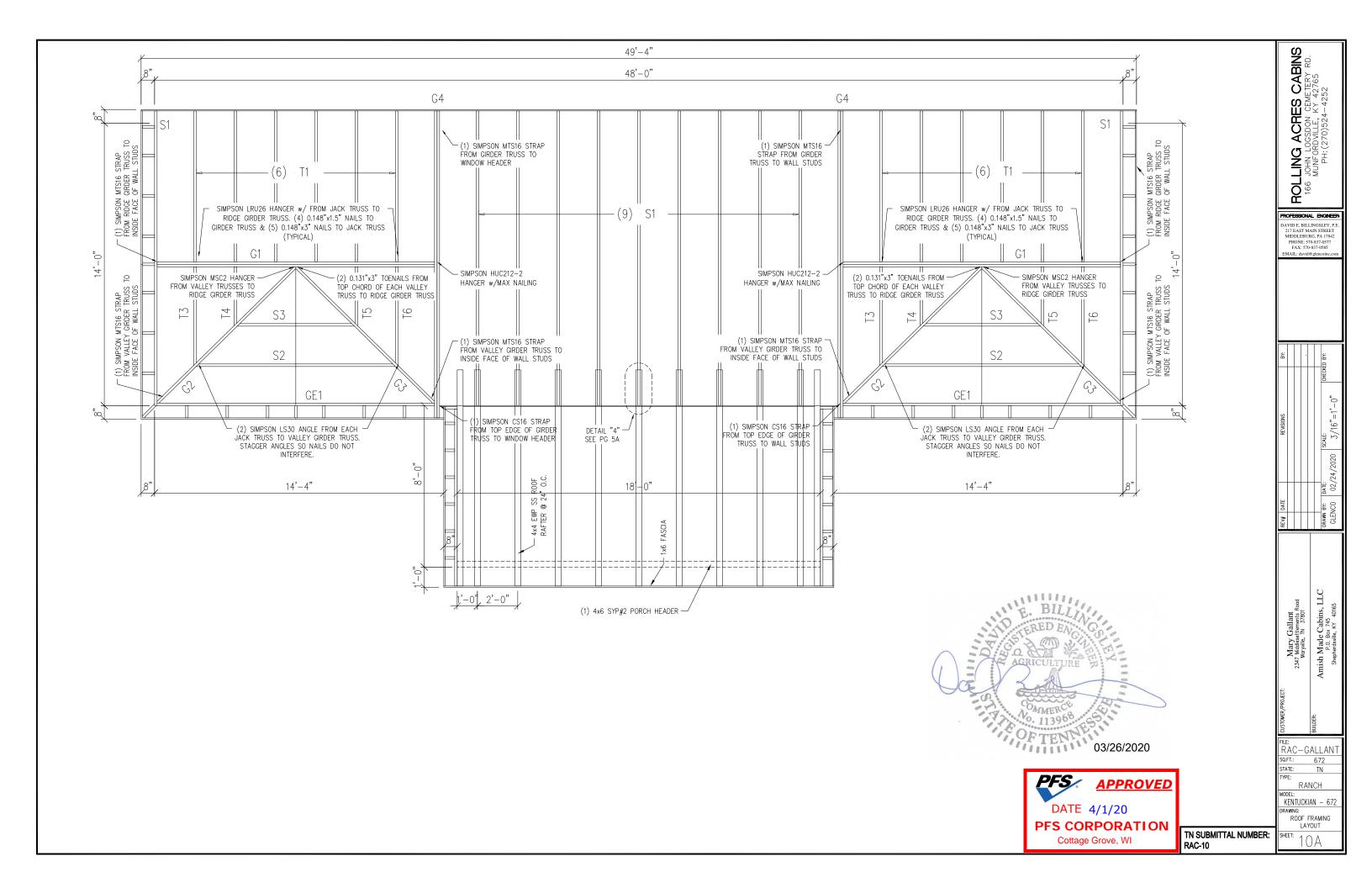
RANCH

KENTUCKIAN - 672

RAWING:
FLOOR FRAMING
LAYOUT

TN SUBMITTAL NUMBER: RAC-10

Cottage Grove, WI



EXTERIOR DOOR SCHEDULE

BUILDER / MANUFACTURER TO VERIFY ALL DOOR ROUGH OPENINGS PRIOR TO CONSTRUCTION

UNIT SIZE	CALL OUT	MFGR / SERIES	F	R.O.	AREA	LIGHT	VENT	U VALUE	SHGC	DP RATING	HARDWARE
36" x 80"	TT-3/0 HALF LITE	THERMA-TRU INSWING	38.38	38.38 x 82.50		5.15	19.38	0.23	0.09	DP45	

"U" and SHGC values as shown my not accurately reflect every situation. Values should be verifed and corrected (if necessary) based on glass type, grilles (if applicable) climate zone, energy star rating, etc.

	WINDOW SCHEDULE												
	BUILDER / MANUFACTURER TO VERIFY ALL WINDOW ROUGH OPENINGS PRIOR TO CONSTRUCTION												
CALL OUT	CALL OUT MFGR R.O. AREA LIGHT VENT U VALUE DP RATING SHGC												
2432	2432 WINDOW MART (7100 SH SERIES) 24 x 32 5.33 3.89 2.17 0.27 DP35 0.26												
3662E	WINDOW MART (7100 SH SERIES)	36	х	62	15.50	13.48	6.74	0.27	DP35	0.26			
2418	WINDOW MART (7100 SH SERIES)	24	х	18	3.00	2.478	1.239	0.27	DP35	0.26			

REQUIRED WINDOW AND DOOR DP RATINGS: INTERIOR ZONES: DP35

NOTE:

- 1. WINDOWS, SKYLIGHTS, AND DOORS OF EQUAL OR BETTER U VALUE, AND MANUFACTURED BY OTHER THAN SHOWN MAY BE SUBSTITUTED.
- 2. WINDOW AREA OF HABITABLE SPACE (LIVING, DINING, INCLUDING KITCHEN DINING IF NO OTHER DINING & SLEEPING SPACE) SHALL BE 8% OF FLOOR AREA FOR NATURAL LIGHT WITH 4% OPENING FOR VENTILATION MINIMUM. IF NOT VENTED MECHANICALLY TO EXTERIOR, BATHROOMS & TOILET ROOMS SHALL HAVE A MINIMUM OPENABLE AREA OF 1 1/2 SQ. FT., 3 SQ. FT. FOR KITCHENETTES & KITCHENS (60 SQ. FT. OR MORE) SHALL HAVE 4% OF FLOOR AREA OPENING FOR MINIMUM VENTILATION.
- 3. EGRESS WINDOWS MUST MEET THE FOLLOWING CRITERIA:

GRADE LEVEL FLOORS:

MIN 5 SQ.FT. OPENING BY NORMAL OPERATION MIN. OPENING OF 24" HIGH - 20" WIDE

MAXIMUM SILL HEIGHT OF 44" FROM SILL TO FLOOR

FLOORS ABOVE GRADE:

OPEN TO 5.7 SQ. FT. BY NORMAL OPERATION

MIN. OPENING OF 24" HIGH - 20" WIDE

MAXIMUM SILL HEIGHT OF 44" FROM SILL TO FLOOR

- 4. REMOVING SASH, TILTING/TWISTING SASH, SNAP AWAY HARDWARE, OR ETC. IS NOT PERMITTED.
- WINDOW AND DOOR SIZES MUST MEET LIGHT, VENTILATION, & EGRESS REQUIREMENTS OF STATE & LOCAL CODES.
- 6. SKYLIGHTS SHALL CONFORM TO SECTION R308.6 OF THE IRC.
- REFERENCE NOTE 11 IN GENERAL NOTES, SHEET 12, FOR COMPLETE LIST OF LOCATIONS WHERE SAFETY GLAZING (TEMPERED GLASS) IS REQUIRED.
- 8. WINDOW AND DOOR FLASHING TO BÉ INSTALLED PER THE MANUFACTURERS INSTALLATION INSTRUCTIONS.





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| DRAWN BY: | DATE: | SCALE: | CHECKET | CHECK

FILE:
RAC-GALLANT
SQ.FT.: 672
STATE: TN
TYPE:

TYPE:

RANCH

MODEL:

KENTUCKIAN - 672

DRAWING:
WINDOW & DOOR
SCHEDULE
SHEET:

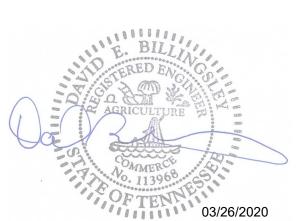
GENERAL NOTES:

- 1. 1. FLOOR PLANS MAY BE FLIPPED, MIRRORED OR BOTH.
- 2. THE MAIN EGRESS EXIT MUST BE A MIN. OF $3'-0" \times 6'-8"$ AND SHALL BE SIDE HINGED.
- 3. EXTERIOR DOORS SHALL NOT SWING OVER STAIRS WHEN A LANDING IS NOT AVAILABLE. EXCEPTION: STORM—SCREEN DOORS ARE PERMITTED TO SWING OVER STEPS.
- 4. ALL NOTES PERTAINING TO "IN FIELD OR ON SITE" ARE BUILDER/ CONTRACTOR RESPONSIBILITIES.
- 5. A SMOKE DETECTOR SHALL BE INSTALLED IN EACH SLEEPING ROOM, OUTSIDE IN THE IMMEDIATE VICINITY OF THE BEDROOMS, AND ON EACH ADDITIONAL STORY OF THE DWELLING INCLUDING BASEMENTS.
- 6. HOLES, OPENINGS, AND ACCESS PROVISIONS FOR COMPLETION OR INSTALLATION OF EQUIPMENT MAY BE COMPLETED IN FIELD, IF DONE IN SUCH A MANNER AS NOT TO AFFECT THE INTEGRITY OF THE STRUCTURE.
- 7. ALL PIPING AND DUCTWORK IN UNHEATED SPACES IS TO BE INSULATED IN COMPLIANCE WITH STATE AND LOCAL CODE REQUIREMENTS.
- 8. ROOF COVERINGS SHALL BE LISTED AS CLASS "C" OR BETTER.
- 9. GUTTERS, LEADERS, AND SPLASH BLOCKS FURNISHED AND INSTALLED BY OTHERS ON SITE.
- 10. APPROVED WATER RESISTIVE BARRIER REQUIRED UNDER ALL SIDING TYPES.

 BARRIER SHALL BE INSTALLED HORIZONTALLY WITH 2 IN. MIN. UPPER LAYER

 OVERLAP OVER LOWER LAYER.
- 11. SAFETY GLAZING (TEMPERED GLASS) REQUIRED IN THE FOLLOWING LOCATIONS:
- 11.1. GLAZING IN SWINGING DOORS EXCEPT JALOUSIES.
- 11.2. GLAZING IN FIXED AND SLIDING PANELS OF SLIDING DOOR ASSEMBLIES AND PANELS IN SLIDING AND BIFOLD CLOSET DOOR ASSEMBLIES.
- 11.3. GLAZING IN STORM DOORS
- 11.4. GLAZING IN DOORS AND ENCLOSURES FOR SHOWERS OR TUBS/ WHIRLPOOLS, UNLESS THE BOTTOM EDGE OF THE GLAZING IS MORE THAN 60" ABOVE THE STANDING OR WALKING SURFACE.
- 11.5. GLAZING WITHIN A 24" RADIUS OF A DOOR PANEL EDGE IN THE CLOSED POSITION AND LESS THAN 60" VERTICALLY ABOVE THE WALKING SURFACE.
- 11.6. GLAZING ADJACENT TO STAIRS, LANDINGS, AND RAMPS WITHIN 36"
 HORIZONTALLY OF A WALKING SURFACE AND LESS THEN 60" ABOVE WALKING SURFACE
- 11.7. GLAZING ADJACENT TO STAIRS WITHIN 60" OF BOTTOM TREAD AND LESS THEN 60" ABOVE WALKING SURFACE.
- 11.8. GLAZING IN AN INDIVIDUAL FIXED OR OPERABLE PANEL (MUST MEET ALL THE FOLLOWING):
- 11.8.1. INDIVIDUAL PANE GREATER THEN 9 SQ.FT.
- 11.8.2. BOTTOM EDGE OF GLAZING LESS TEN 18" ABOVE FLOOR
- 11.8.3. TOP EDGE GREATER THEN 36" ABOVE FLOOR.
- 11.8.4. ONE OR MORE WALKING SURFACES WITHIN 36" HORIZONTALLY OF THE GLAZING.

- 12. ANY STRUCTURAL GLUED LAMINATED TIMBERS THAT ARE EXPOSED MUST BE PRESSURE TREATED.
- 13. WHERE LEDGERS ARE REQUIRED, A MIN. 2x2 LEDGER MUST BE USED, OR A JOIST HANGER IN PLACE OF LEDGER.
- 14. SPACING OF INTERMEDIATE GUARDRAILS AT STAIRWAYS SHALL BE SUCH THAT A 4" SPHERE CANNOT PASS THROUGH.
- 15. ANY DECK, PORCH, RAMP, OR STEPS, REACHING A HEIGHT GREATER THAN 30" ABOVE THE FLOOR OR GRADE ARE REQUIRED TO HAVE GUARDS AT LEAST 36" ABOVE THE WALKING SURFACE.
- 16. CAULKING AND SEALANTS IN THE EXTERIOR ENVELOPE MUST BE IN COMPLIANCE WITH ENERGY CONSERVATION CONSTRUCTION CODE. ALL JOINTS, CONNECTIONS, AND PENETRATIONS BETWEEN CONDITIONED SPACE AND UNCONDITIONED OR OUTSIDE SPACE MUST BE SEALED.
- 17. MIN. FLAME SPREAD CLASSIFICATION OF ALL FINISH MATERIALS SHALL HAVE A RATING OF LESS THAN 200.
- 18. CONNECTORS OF EQUAL OR GREATER CAPACITIES MAY BE SUBSTITUTED FOR SIMPSON CONNECTORS.





ROLLING ACRES CABINS 166 JOHN LOGSDON CEMETERY RD. MUNFORDYILLE, KY 42765 PH: (270)524-4252

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					SCALE:	
					DATE:	GLENCO 02/24/2020
					DRAWN BY: DATE:	GLENCO
Mary Gallant	2347 Middlesettlements Road	Maryville, TN 37801		BUILDER:	Almsh Made Cabins, LLC	Shepherdsville, KY 40165
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RANCH

KENTUCKIAN - 672

GENERAL NOTES

IRC 2006 - TABLE R602.3(1) FASTENER SC	CHEDULE FOR STRUCT	URAL MEMBERS
DESCRIPTION OF BUILDING ELEMENTS	NUMBER AND TYPE OF FASTENERa,b,c	SPACING OF FASTENERS
Joist to sill or girder, toe nail	3-8d (2 ¹ / ₂ " x 0.113")	_
	2-8d (2 ¹ / ₂ " x 0.113")	
1" x 6" subfloor or less to each joist, face nail	2 staples, $1^{3}/_{4}$ "	_
2" subfloor to joist or girder, blind and face nail	2-16d (3 ¹ / ₂ " x 0.135")	_
Sole plate to joist or blocking, face nail	16d (3 ¹ / ₂ " x 0.135")	16" o.c.
Top or sole plate to stud, end nail	2-16d (3 ¹ / ₂ " x 0.135")	_
	3-8d (2 ¹ / ₂ " x 0.113") or	
Stud to sole plate, toe nail	2-16d (3 ¹ / ₂ " x 0.135")	_
Double studs, face nail	10d (3" x 0.128")	24" o.c.
Double top plates, face nail	10d (3" x 0.128")	24" o.c.
Sole plate to joist or blocking at braced wall panels	3-16d (3 ¹ / ₂ " x 0.135")	16" o.c.
Double top plates, minimum 24-inch offset of end joints,	1	_
face nail in lapped area	8-16d (3 ¹ / ₂ " x 0.135")	
Blocking between joists or rafters to top plate, toe nail	3-8d (2 ¹ / ₂ " x 0.113")	_
Rim joist to top plate, toe nail	8d (2 ¹ / ₂ " x 0.113")	6" o.c.
Top plates, laps at corners and intersections, face nail	2-10d (3" x 0.128")	_
Built-up header, two pieces with ¹ / ₂ " spacer	16d (3 ¹ / ₂ " x 0.135")	16" o.c. along each edge
Continued header, two pieces	16d (3 ¹ / ₂ " x 0.135")	16" o.c. along each edge
Ceiling joists to plate, toe nail	3-8d (2 ¹ / ₂ " x 0.113")	_
Continuous header to stud, toe nail	4-8d (2 ¹ / ₂ " x 0.113")	_
Ceiling joist, laps over partitions, face nail	3-10d (3" x 0.128")	_
Ceiling joist to parallel rafters, face nail	3-10d (3" x 0.128")	_
Rafter to plate, toe nail	2-16d (3 ¹ / ₂ " x 0.135")	_
1" brace to each stud and plate, face nail	2-8d (2 ¹ / ₂ " x 0.113")	_
2 State to each state and place, rate han	2 staples, 1³/ ₄ "	
1" x 6" sheathing to each bearing, face nail	2-8d (2 ¹ / ₂ " x 0.113")	
1 A 0 Sheathing to each bearing, face han	2 staples, 1³/ ₄ "	
1" x 8" sheathing to each bearing, face nail	2-8d (2 ¹ / ₂ " x 0.113")	
1 x 8 sheathing to each bearing, race half	3 staples, 1 ³ / ₄	_
Midanthau 111 · Oll de athire te and benefit for a sil	3-8d (2 ¹ / ₂ " x 0.113")	
Wider than 1" x 8" sheathing to each bearing, face nail	4 staples, 1 ³ / ₄ "	_
Built-up corner studs	10d (3" x 0.128")	24" o.c.
Duilt up girdous and booms 2 in the home boundaries	104 (2" < 0.120")	Nail each layer as follows: 32" o.c. at top and bottom and staggered. Two nails
Built-up girders and beams, 2-inch lumber layers	10d (3" x 0.128")	at ends and at each splice. At each bearing
2" planks	2-16d (3 ¹ / ₂ " x 0.135")	Aceach bearing
Roof rafters to ridge, valley or hip rafters: toe nail	4-16d (3 ¹ / ₂ " x 0.135")	_
face nail	4-16d (3 / ₂ " x 0.135") 3-16d (3 ¹ / ₂ " x 0.135")	
	3-16d (3 $/_2$ " x 0.135") 3-8d (2 $^1/_2$ " x 0.113")	
Rafter ties to rafters, face nail		_
Collar tie to rafter, face nail, or $1^{1}/_{4}$ " x 20 gage ridge strap	3-10d (3" x 0.128")	

IRC 2006 - TABLE R602.3(1)—continued FASTENER SCHEDULE FOR							
	STRUCTURAL MEMBERS						
		SPACING (OF FASTENERS				
DESCRIPTION OF BUILDING MATERIALS	DESCRIPTION OF FASTENERB, c, e	Edges (inches)i	Intermediate supports ^{c, e} (inches)				
Wood structural panels, su	bfloor, roof and wall sheathing to framing sheathing to framing		leboard wall				
5/16" - 1/2"	6d common (2" × 0.113") nail (subfloor, wall) 8d common (2 ¹ / ₂ " × 0.131") nail (roof)f	6	12 ^g				
¹⁹ / ₃₂ " - 1"	8d common nail (2 ¹ / ₂ " × 0.131")	6	12 ^g				
1 ¹ / ₈ " - 1 ¹ / ₄ "	10d commor (3" × 0.148") nail or 8d (2 ¹ / ₂ " × 0.131") deformed nail	6	12				
	Other wall sheathing ^h	l					
	1 ¹ / ₂ " galvanized roofing nail 8d common						
¹ / ₂ " structural cellulosic fiberboard sheathing	$(2^{1}/_{2}" \times 0.131")$ nail; staple 16 ga., $1^{1}/_{2}"$ long	3	6				
25	1 ³ / ₄ " galvanized roofing nail 8d common		6				
²⁵ / ₃₂ " structural cellulosic fiberboard sheathing	$(2^{1}/_{2}" \times 0.131")$ nail; staple 16 ga., $1^{3}/_{4}"$ long	3					
	1 ¹ / ₂ " galvanized roofing nail;						
¹ / ₂ " gypsum sheathing ^d	6d common (2" × 0.131") nail; staple galvanized	4	8				
	$1^1/_2$ " long; $1^1/_4$ " screws, Type W or S						
⁵ / ₈ " gypsum sheathing ^d	$1^3/_4$ " galvanized roofing nail; 8d common ($2^1/_2$ " × 0.131") nail; staple galvanized $1^5/_8$ " long; $1^5/_8$ " screws, Type W or S	4	8				
Wood structural	panels, combination subfloor underlayme	nt to frami	ng				
	6d deformed (2" × 0.120") nail or						
³ / ₄ " and less	8d common (2 ¹ / ₂ " × 0.131") nail	6	12				
⁷ / ₈ " - 1"	8d common $(2^1/2^n \times 0.131^n)$ nail or 8d deformed $(2^1/2^n \times 0.120^n)$ nail	6	12				
	10d commor (3" × 0.148") nail or	-					

For SI: 1 inch = 25.4 mm, 1 foot = 304.8 mm, 1 mile per hour = 0.447 m/s; 1ksi = 6.895 MPa.
a. All nails are smooth-common, box or deformed shanks except where otherwise stated. Nails used for framing and sheathing connections shall have minimum average bending yield strengths as shown: 80 ksi for shank diameter of 0.192 inch (20d common nail), 90 ksi for shank diameters larger than 0.142 inch but not larger than 0.177 inch, and 100 ksi for shank diameters of 0.142 inch or less.

b. Staples are 16 gage wire and have a minimum $^7/_{16}$ -inch on diameter crown width. c. Nails shall be spaced at not more than 6 inches on center at all supports where spans are 48

inches or greater.

d. Four-foot by 8-foot or 4-foot by 9-foot panels shall be applied vertically

d. Four-foot by 8-foot or 4-foot by 9-foot panels shall be applied vertically.

e. Spacing of fasteners not included in this table shall be based on Table R602.3(2).

f. For regions having basic wind speed of 110 mph or greater, 8d deformed $(2^1/2^n \times 0.120)$ nails shall be used for attaching plywood and wood structural panel roof sheathing to framing within minimum 48-inch distance from gable end walls, if mean roof height is more than 25 feet, up to 35 feet maximum.

g. For regions having basic wind speed of 100 mph or less, nails for attaching wood structural panel roof sheathing to gable end wall framing shall be spaced 6 inches on center. When basic wind speed is greater than 100 mph, nails for attaching panel roof sheathing to intermediate supports shall be spaced 6 inches on center for minimum 48-inch distance from ridges, eaves and gable end walls; and 4 inches on center to gable end wall framing.

h. Gypsum sheathing shall conform to ASTM C79 and shall be installed in accordance with GA 253. Fiberboard sheathing shall conform to either ASTM C 208.

i. Spacing of fasteners on floor sheathing panel edges applies to panel edges supported by framing members and require blocking and at all floor perimeters only. Spacing of fasteners on roof sheathing panel edges applies to panel edges supported by framing members and require blocking. Blocking of roof or floor sheathing panel edges perpendicular to the framing members need not be provided except as required by other provisions of this code. Floor perimeter shall be supported by framing members or solid blocking.



03/26/2020

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						DRAWN BY: DATE: SCALE:	GLENCO 02/24/2020 N.
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RAC-GALLANT

RANCH
MODEL:
KENTUCKIAN — 672
PRAWING:
FASTENING SCHEDULE

672 TN

	DESCRIPTION ^{a, b} OF FASTENER AND	SPAC	ING ^c OF FASTENERS
NOMINAL MATERIAL	LENGTH	Edges	Intermediate suppo
THICKNESS (inches)	(inches)	(inches)	(inches)
Wood structural par	els subfloor, roof and wall sheathing	to framing	and particleboard wal
	sheathing to framing		
1 -	Staple 15 ga. 1 ³ / ₄	4	8
up to ¹ / ₂	0.097 - 0.099 Nail 2 ¹ / ₄	3	6
	Staple 16 ga. 1 ³ / ₄	3	6
-	0.113 Nail 2	3	6
$19/_{32}$ and $^{5}/_{8}$	Staple 15 and 16 ga. 2	4	8
	0.097 - 0.099 Nail 2 ¹ / ₄	4	8
	Staple 14 ga. 2	4	8
$^{23}/_{32}$ and $^{3}/_{4}$	Staple 15 ga. 1 ³ / ₄	3	6
732 74	0.097 - 0.099 Nail 2 ¹ / ₄	4	8
	Staple 16 ga. 2	4	8
	Staple 14 ga. 2 ¹ / ₄	4	8
1	0.113 Nail 2 ¹ / ₄	3	6
_	Staple 15 ga. 2 ¹ / ₄	4	8
	0.097 - 0.099 Nail 2 ¹ / ₂	4	8
NOMINAL MATERIAL	DESCRIPTION ^{a,b} OF FASTENER AND	SPAC	ING ^c OF FASTENERS
THICKNESS	LENGTH	Edges	Body of panel ^d
(inches)	(inches)	(inches)	(inches)
Flo	or underlayment; plywood-hardboar	d-particleb	oard ^f
	Plywood		
	1 ¹ / ₄ ring or screw shank		
¹ / ₄ and ⁵ / ₁₆	nail—minimum	3	6
74 dila 716	12 ¹ / ₂ ga. (0.099") shank diameter		
	Staple 18 ga., $\frac{7}{8}$, $\frac{3}{16}$ crown width	2	5
$^{1}/_{32}$, $^{3}/_{8}$, $^{15}/_{32}$, $^{1}/_{2}$ and	1 ¹ / ₄ ring or screw shank		
¹⁹ / ₃₂	nail—minimum	6	8 ^e
732	$12^{1}/_{2}$ ga. (0.099") shank diameter		
	1 ¹ / ₂ ring or screw shank		
$^{5}/_{8}$, $^{23}/_{32}$ and $^{3}/_{4}$	nail—minimum		8
	$12^{1}/_{2}$ ga. (0.099") shank diameter	6	
707 752 74	Staple 16 ga. 1 ¹ / ₂	6	8
7 07 7 32 7 4	Staple 16 ga. 1 /2		
	Hard b oard ^f		
107 132 74	L	6	6
	Hardboard [†] 1 ¹ / ₂ long ring-grooved underlayment nail	6	6
0.200	Hardboard ^f 1 ¹ / ₂ long ring-grooved underlayment nail 4d cement-coated sinker nail	6	6 6
	Hardboard ^f 1 ¹ / ₂ long ring-grooved underlayment nail 4d cement-coated sinker nail Staple 18 ga., ⁷ / ₈ long (plastic		
	Hardboard ^f 1 ¹ / ₂ long ring-grooved underlayment nail 4d cement-coated sinker nail Staple 18 ga., ⁷ / ₈ long (plastic coated)	6	6
	Hardboard ^f 1 ¹ / ₂ long ring-grooved underlayment nail 4d cement-coated sinker nail Staple 18 ga., ⁷ / ₈ long (plastic coated) Particleboard	6	6
	Hardboard ^f 1 ¹ / ₂ long ring-grooved underlayment nail 4d cement-coated sinker nail Staple 18 ga., ⁷ / ₈ long (plastic coated) Particleboard 4d ring-grooved underlayment nail	3	6
0.200	Hardboard ^f 1 ¹ / ₂ long ring-grooved underlayment nail 4d cement-coated sinker nail Staple 18 ga., ⁷ / ₈ long (plastic coated) Particleboard 4d ring-grooved underlayment nail Staple 18 ga., ⁷ / ₈ long, ³ / ₁₆ crown	3 3 3	6 6 6
0.200	Hardboard ^f 1 ¹ / ₂ long ring-grooved underlayment nail 4d cement-coated sinker nail Staple 18 ga., ⁷ / ₈ long (plastic coated) Particleboard 4d ring-grooved underlayment nail Staple 18 ga., ⁷ / ₈ long, ³ / ₁₆ crown 6d ring-grooved underlayment nail	6 3 3 3 6	6 6 6 6
0.200	Hardboard 1 ¹ / ₂ long ring-grooved underlayment nail 4d cement-coated sinker nail Staple 18 ga., ⁷ / ₈ long (plastic coated) Particleboard 4d ring-grooved underlayment nail Staple 18 ga., ⁷ / ₈ long, ³ / ₁₆ crown 6d ring-grooved underlayment nail Staple 16 ga., 1 ¹ / ₈ long, ³ / ₈ crown	6 3 3 3 6 3	6 6 6 6 10 6
0.200	Hardboard ^f 1 ¹ / ₂ long ring-grooved underlayment nail 4d cement-coated sinker nail Staple 18 ga., ⁷ / ₈ long (plastic coated) Particleboard 4d ring-grooved underlayment nail Staple 18 ga., ⁷ / ₈ long, ³ / ₁₆ crown 6d ring-grooved underlayment nail	6 3 3 3 6	6 6 6 6

- b. Staples shall have a minimum crown width of $^{7}/_{16}$ -inch on diameter except as noted.
- c. Nails or staples shall be spaced at not more than 6 inches on center at all supports where spans are 48 inches or greater. Nails or staples shall be spaced at not more than 12 inches on center at intermediate supports for floors.
- d. Fasteners shall be placed in a grid pattern throughout the body of the panel.
- e. For 5-ply panels, intermediate nails shall be spaced not more than 12 inches on center each way f. Hardboard underlayment shall conform to ANSI/AHA A135.4.

2006 IRC TABLE R702.3.5 MINIMUM THICKNESS AND APPLICATION OF GYPSUM BOARD

THICKNESS OF GYPSUM BOARD	APPLICATI ON	ORIENTATION OF GYPSUM BOARD TO FRAMING	MAXIMUM SPACING OF FRAMING MEMBERS	MAXIMUM SPACING OF FASTENERS (inches)		SIZE OF NAILS FOR APPLICATION MAGRICULTUM MAGRICULTUM
(inches)			(inches	Nails ^a	Screws ^b	TO WOOD FRAMING ^c
	•	T		Applica	ation withou	t adhesive
3/8"	Ceiling ^d	Perpendicular	16	7	12	13 gage, $1^{1}/_{4}$ " long, $^{19}/_{64}$ " head; 0.098" diameter, $1^{1}/_{4}$ " long, annular-ringed
,	Wall	Either direction	16	8	16	or 4d cooler nail, $0.080''$ diameter, $1^3/8''$ long, $7/32''$ head.
	Ceiling	Either direction	16	7	12	13 gage, $1^3/8$ " long, $1^9/64$ " head; 0.098" diameter, $1^1/4$ " long, annular-ringed
1/2"	Ceiling ^d	Perpendicular	24	7	12	5d cooler nail, 0.086" diameter, $1^5/8$ " long, $1^5/64$ " head; or gypsum board
	Wall	Either direction	24	8	12	nail, $0.086''$ diameter, $1^{5}/8''$ long, $9/32''$ head.
	Wall	Either direction	16	8	16	,,
	Ceiling	Either direction	16	7	12	13 gage, $1^{5}/8$ " long, $^{19}/64$ " head; 0.098" diameter, $1^{3}/8$ " long, annular-ringed
5/8"	5/8" Ceiling ^e P	Perpendicular	24	7	12	$\begin{bmatrix} 1 & 1 & 1 \\ 6 & 1 & 1 \end{bmatrix}$ diameter, $1^{7}/8$ long, $1^{7}/4$ head; or gypsum board nail
	Wall	Either direction	24	8	12	$0.0915''$ diameter, $1^7/8''$ long, $^{19}/_{64}''$ head.
	Wall	Either direction	16	8	16	, , , , , , , , , , , , , , , , , , ,
				Appli	cation with a	adhesive
3/8"	Ceiling ^d	Perpendicular	16	16	16	Same as above for $^3/_8$ " gypsum board
	Wall	Either direction	16	16	24	
1, 5,	Ceiling	Either direction	16	16	16	
$^{1}/_{2} \text{ or } ^{5}/_{8}$	Ceiling ^d	Perpendicular	24	12	16	Same as above for ¹ / ₂ " and ⁵ / ₈ " gypsum board, respectively
	Wall	Either direction	24	16	24	
Two	Ceiling	Perpendicular	16	16	16	Base ply nailed as above for $^{1}/_{2}$ " gypsum board; face ply installed with
³ / ₈ layers	Wall	Either direction	24	24	24	adhesive
For SI: 1 inch	= 25.4 mm.					
a. For applica	tion without	adhesive, a pair of i	nails spaced n	ot less than 2	inches apart	or more than $2^{1}/_{2}$ inches apart may be used with the pair of nails spaced 12

a. For application without adhesive, a pair of nails spaced not less than 2 inches apart or more than 2 1/2 inches apart may be used with the pair of nails spaced 12 inches on center.

c. Where cold-formed steel framing is used with a clinching design to receive nails by two edges of metal, the nails shall be not less than 5/8 inch longer than the gypsum board thickness and shall have ringed shanks. Where the cold-formed steel framing has a nailing groove formed to barbed shanks or be 5d, $13^{1}/_{2}$ gage, $^{15}/_{8}$ inches long, $^{15}/_{64}$ -inch head for $^{1}/_{2}$ -inch gypsum board; and 6d, 13 gage, $^{17}/_{8}$ inches long, $^{15}/_{64}$ -inch head for $^{1}/_{2}$ -inch gypsum board; and 6d, 13 gage, $^{17}/_{8}$ inches long, $^{15}/_{64}$ -inch head for $^{1}/_{2}$ -inch gypsum board; and 6d, 13 gage, $^{17}/_{8}$ inches long, $^{15}/_{64}$ -inch head for $^{1}/_{2}$ -inch gypsum board; and 6d, 13 gage, $^{17}/_{8}$ inches long, $^{15}/_{64}$ -inch head for $^{1}/_{2}$ -inch gypsum board; and 6d, 13 gage, $^{17}/_{8}$ inches long, $^{15}/_{64}$ -inch head for $^{1}/_{2}$ -inch gypsum board; and 6d, 13 gage, $^{17}/_{8}$ inches long, $^{15}/_{64}$ -inch head for $^{1}/_{2}$ -inch gypsum board; and 6d, 13 gage, $^{17}/_{8}$ inches long, $^{15}/_{64}$ -inch head for $^{1}/_{2}$ -inch gypsum board; and 6d, 13 gage, $^{17}/_{8}$ inches long, $^{15}/_{8}$ inches long, $^{15}/_{64}$ -inch head for $^{1}/_{2}$ -inch gypsum board; and $^{17}/_{8}$ inches long, $^{15}/_{8}$ inches long, $^{15}/_{8}$ inches long, $^{15}/_{64}$ -inch head for $^{1}/_{2}$ -inch gypsum board; and $^{17}/_{8}$ inches long, $^{15}/_{8}$ inches

d. Three-eighths-inch-thick single-ply gypsum board shall not be used on a ceiling where a water-based textured finish is to be applied, or where it will be required to support insulation above a ceiling. On ceiling applications to receive a water-based texture material, either hand or spray applied, the gypsum board shall be applied perpendicular to framing. When applying a water-based texture material, the minimum gypsum board thickness shall be increased from for 16-inch on center framing, and from $^{1}/_{2}$ inch to $^{5}/_{8}$ inch for 24-inch on center framing or $^{1}/_{2}$ -inch sag-resistant gypsum ceiling board shall be used.

e. Type X gypsum board for garage ceilings beneath habitable rooms shall be installed perpendicular to the ceiling framing and shall be fastened at maximum 6 inches o.c. by minimum $exttt{1}^7/ exttt{8}$ inches 6d coated nails or equivalent drywall screws.

ROLLING ACRES CABINS
166 JOHN LOGSDON CEMETERY RD.
MUNICORDWILE, KY 42765

03/26/2020

AVID E. BILLINGSLEY, 217 EAST MAIN STREE' MIDDLEBURG, PA 1784 PHONE: 570-837-0577 FAX: 570-837-0585

FASTENING SCHEDULE

RAC-GALLANT

RANCH

KENTUCKIAN - 672

672





Tennessee Modular Building Unit Minimum General Plan Set Requirements Checklist One and Two Family Residential Only



Every page of the submittal must be wet stamped, signed, and dated by an architect or engineer duly registered and in good standing by the State of Tennessee. Wet seal every page of every set. This applies to any and all plan submittals.

Every page of the submittal must be reviewed, approved and wet stamped by the third party design review agency. Wet approval stamp must be on every page of every set. This applies to any and all plan submittals.

Submittal Requirements:

- 1. One (1) copy of all submitted paperwork
- 2. Check and Tennessee Modular Building Unit-Model Plans submittal form. Form must be completely filled out and signed. Provide complete description and actual overall unit dimensions.
- 3. Check number is listed on the submittal form.
- 4. Plan submittal number is listed on the check @ "For" line.

Model Plans Submittal Requirements:

Cover Sheet:

1. X All pages have Tennessee Modular Plan Number. This number must appear on all other documents including submittal form.

Tennessee uses special numbering system that must be on the plans on all pages.

Manufactures initials- Plan # xyz

Example XYZ-012

2. X The Cover Sheet must list the current adopted codes.

2006 International Residential Code for one and two family dwellings

2008 National electrical Code NFPA 70.

2006 International Energy Conservation Code

Safety Glazing Materials Tennessee Code Annotated, Title 68, Chapter 120, Part 3 (T.C.A. 68-120-301)

- 3. X Correct Construction Type: Wood Frame- Unprotected
- 4. Correct occupancy group: Residential 1 and 2 Family Dwelling

- 5. X Number of Stories.
- 6. X Indicate whether the building is Sprinklered or Unsprinklered
- 7. Indicate the seismic zone the buildings is designed for: (A,B,C,Do, D1,D2,E)
- 8. X Indicated building design Live loads: Roof, Floor, Stairs Cuarhand-railings.
- 9. \times Provide Wind speed design velocity: 90 mph 3 sec gust
- 10. X Indicate Height of building.
- 11. X Indicated area of completed modular building.
- 12. X Indicate Ground Snow Load.
- 13. X Table of Contents of all submitted information numbered and indexed.

DATE 4/1/20

PFS CORPORATION

Cottage Grove, WI

14. X Notice to Local Authority Inspection Departments regarding items to be site installed and inspected. List all items to be site installed.

General Model Plan Requirements:

- 1. Check to make sure that all of the minimum required pages are included prior to submittal.
- 2. X Manufacturer's name and facility address on all submitted data.
- 3. X Architect/Engineers name and office address on all submitted data.
- 4. X Floor Plans must include location of TN Modular Building Certification Labels. (Labels must be installed on a permanent, non-removable surface) (not on Panel cover) refer to rule 0780-02-13-11. Each transportable section must be labeled.
- 5. X Floor plans must include the location of the Data plate.
- 6. X Floor plan must indicate all space names.
- 7. X All spaces to be dimensioned.
- 8. X Provide overall dimensions of the structure.
- Provide complete glazing schedules. (indicate light, ventilation requirements, clear opening dimensions, egress windows and location, and safety glazing)
- 10. X Provide complete door and hardware schedules.
- 11. Provide complete second floor plans, including electric, plumbing, stairs, and mechanical. (Whether factory built, site built or proposed)
- 12. X Building Elevations
- 13. X Provide truss drawings.
 - a. Truss drawings must be wet stamped signed and dated by an engineer duly registered in good standing by the State of Tennessee.
 - b. The truss drawing labeled same as section drawing reference.
 - c. The wind load used in the calculation is same as listed on cover sheet.
 - d. The design criteria is listed as 2006 IRC. No other standard is acceptable.

- e. Provide truss calculations.
- 14. Engineered header and beam drawings
 - Must be wet stamped signed and dated by Duly registered by State of Tennessee engineer.
 - b. Labeled same as floor plan reference.
 - c. Design criteria as referenced in 2006 IRC.
 - d. Provide engineer calculations.
- 15. Section Drawings(s) including stairs to show treads/risers/handrails/headroom. (stair details to be provided for site built stairs also)
- 16. X Attic access location and size.
- 17. X Attic ventilation requirements.
- 18. Plumbing (must be provided on separate sheets)
 - a. Sanitary waste riser diagram(s) list material types being used.
 - b. Domestic water riser diagrams(s). List material types being used.
 - c. Fuel Gas riser diagram(s). List material types being used. (Gas type, inlet pressure, pressure drop)
 - d. Water heater type and capacity and installation details.
- 19. Electrical (must be provided on separate sheets)
 - a. Electrical panel location(s) shown on floor plans. Provide service entrance sizes.
 - b. Electrical panel schedule(s)
 - c. Electrical Load calculations.
 - d. Location of electrical boxes, provide electrical fixture schedule.
 - e. Provide smoke detector locations.
 - f. Location of exterior lighting and receptacles.
- 20. Site connection details and instructions (including factory built dormers etc.)
 - a. X Cross section (s) drawings(s).
 - i. Floor
 - 1. Floor joist and beam sizes and spacings
 - 2. Material species and grade
 - 3. Sheathing and decking
 - 4. Fastening types and instructions
 - 5. Insulation types
 - 6. Additional details for clarification



ii. Wall

- 1. Stud and column sizes and spacing
- 2. Materials species and grade

- 3. Sheathing and bracing
- 4. Headers and lintels
- 5. Fastening types and instructions.
- 6. Insulation types
- 7. Finishes
- iii. Ceiling and roof:
 - 1. Truss, rafter and beam spacing
 - 2. Material species and grade
 - 3. Sheathing and decking
 - 4. Fastening
 - 5. Insulation types
 - 6. Finishes
 - 7. Provide details on dormers. Dormers are required to be labeled if they are constructed in manufacturing facility.
- iv. Fire-blocking/Firestopping
- 21. Proposed foundation plan and details (must be provided on separate sheets)
 - a. Structural calculations.
 - b. Crawl space ventilation requirements location and size
 - c. Crawl space access size and location.
 - d. Footings, pier and curtain wall locations. Provide dimensioned cross sections details and specifications.
 - e. Mortar types
 - f. Minimum concrete compressive strength.
 - g. Design soil bearing capacity
 - h. Provide anchorage information
 - i. Sill plate to piers and curtain wall.
 - ii. Modular building to sill plate
 - iii. Engineered tie-down systems and details.
- 22. Mechanical: Must be provide on separate sheets. If register is shown, then complete HVAC system must be designed.
 - a. Provide design calculations
 - b. Provide complete hvac specifications (units, duct, materials, etc)
 - c. Provide installed hvac capacity
 - d. Duct sizes
 - e. Provide supply and return location and sizes.
- 23. All the calculation pages are included in the submittal:
 - a. Heat loss calculations
 - b. Electrical worksheet



- c. Lighting work sheet
- d. Mechanical worksheet
- 24. No changes can be made to the plans by hand unless the changes are clouded, initialed, and dated by the architect/engineer of record.
- 25. All plans must be submitted to and filed by the State of Tennessee prior to the commencement of any construction.

If there are any question on what is required by the Tennessee Modular building Act. Please refer to the Modular Building Laws and Rules at the Tennessee web site.

https://www.tn.gov/commerce/fire/sections-programs/fire-manufactured-housing.html





REScheck Software Version 4.7.0

Compliance Certificate

Project RAC-Gallant / RAC-10

Energy Code: 2009 IECC

Location: Maryville, Tennessee

Construction Type: Single-family
Project Type: New Construction

Conditioned Floor Area: **672 ft2** Glazing Area **11%**

Climate Zone: 4 (3937 HDD)

Permit Date: Permit Number:

Construction Site:

2347 Middlesettlements Road

Maryville, TN 37801

Owner/Agent:

Amish Made Cabins, LLC

PO Box 745

Shepherdsville, KY 40165

AGRICULTURE

OMMERCO

OMMERCO

O3/26/2020

Designer/Contractor: Rolling Acres Cabins

166 John Logsdon Cemetery Rd Munfordville, KY 42765

Compliance: Passes using UA trade-off

Compliance: 24.2% Better Than Code Maximum UA: 178 Your UA: 135

The % Better or Worse Than Code Index reflects how close to compliance the house is based on code trade-off rules. It DOES NOT provide an estimate of energy use or cost relative to a minimum-code home.

Envelope Assemblies

Assembly	Gross Area or Perimeter	Cavity R-Value	Cont. R-Value	U-Factor	UA
Floor 1: All-Wood Joist/Truss:Over Unconditioned Space	672	30.0	0.0	0.033	22
Ceiling 1: Cathedral Ceiling	696	30.0	0.0	0.034	24
Left Wall: Wood Frame, 16" o.c.	112	18.0	0.0	0.062	7
Right Wall: Wood Frame, 16" o.c.	ED 112	18.0	0.0	0.062	7
Front Wall: Wood Frame, 16" o.c. DATE 4/1/20	364	18.0	0.0	0.062	15
3662: Vinyl/Fiberglass Frame:Double Pane with Love PFS CORPORATION)N 16			0.270	4
3662: Vinyl/Fiberglass Frame:Double Pane with Lov-E Cottage Grove, WI	16			0.270	4
3662: Vinyl/Fiberglass Frame:Double Pane with Low-E	16			0.270	4
3662: Vinyl/Fiberglass Frame:Double Pane with Low-E	16			0.270	4
3662: Vinyl/Fiberglass Frame:Double Pane with Low-E	16			0.270	4
3662: Vinyl/Fiberglass Frame:Double Pane with Low-E	16			0.270	4
Door 1: Solid	22			0.230	5
Rear Wall: Wood Frame, 16" o.c.	364	18.0	0.0	0.062	21
2432: Vinyl/Fiberglass Frame:Double Pane with Low-E	5			0.270	1
Door 1: Solid	22			0.230	5
Front Reverse Gable Walls: Wood Frame, 16" o.c.	42	18.0	0.0	0.062	2
2418: Vinyl/Fiberglass Frame:Double Pane with Low-E	3			0.270	1

Project Title: RAC-Gallant / RAC-10

Data filename: G:\01 Drawing File\Rolling Acres\RAC 2020\RAC-Gallant Kentuckian 14x48 (TN)\RAC-

Gallant Kentuckian 14x48 03-03-2020 JJ.rck

Report date: 03/16/20

PAGE-15

Assembly	Gross Area or R-Value Perimeter	Cont. U-Factor R-Value	UA
2418: Vinyl/Fiberglass Frame:Double Pane with Low-E	3	0.270	1

2418: Vinyl/Fiberglass Frame:Double Pane with Low-E

0.270

Compliance Statement: The proposed building design described here is consistent with the building plans, specifications, and other calculations submitted with the permit application. The proposed building has been designed to meet the 2009 IECC requirements in REScheck Version 4.7.0 and to comply with the mandatory requirements listed in the REScheck Inspection Checklist.

Justin Jones

Signaturé

3-16-2020

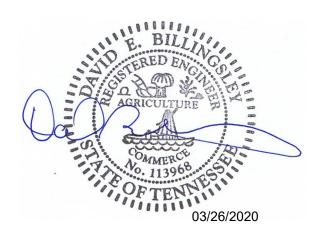
Date

Project Notes:

Name - Title

ROLLING ACRES CABINS 166 JOHN LOGSDON CEMETERY RD MUNFORDVILLE, KY 42765

RAC-10





Project Title: RAC-Gallant / RAC-10 Report date: 03/16/20

electric tankless hot water heater

HomeAdvantage II for Single Point-of-Use or Multiple Applications

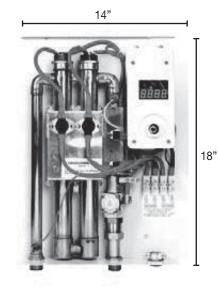
MODELS	VOLTS	kW	AMPS
HA018240	240	18.0	75A

€ Eemax™ Hot Water Solutions Made Easy

APPLICATIONS

Water-Saver Shower Head
 Standard Shower Head
 Standard Hand Sink
 Kitchen Sink
 Bath Tub
 Dishwasher
 Washing Machine
 1.5 GPM
 0.5 GPM
 1 to 2 GPM
 1 to 2 GPM
 1 to 2 GPM

Average Gallons Per Minute (GPM) based on 2010 Plumbing Standards





PERFORMANCE FEATURES

- Instant, consistent and endless hot water
- Compact, resistant and stylish with digital temperature control in increments of 1°F ranging from 80°F to 140°F
- Dimensions: 18" x 14 x 3.75"
- 99.8% energy efficient

- Copper Emerson heating elements with brass top increase durability and are threaded for easy replacement
- Simple Installation: 3/4" NPT water connectors for easy flex hose installation

PRODUCT SPEC	IFICATIONS
18 kw	Size: 18"H x 14"W x 3.75"D Weight: 11.25 lbs.
Rated Pressure:	25psi minimum; 150psi maximum
Certifications:	ETL Listed to UL 499 and CSA
Unique Features:	Solid, hand-welded exchanger NPT adapters included
Standard Temp Settings:	120°F (Adjustable 80-140°F)
Temp Accuracy:	+/- 1° at Steady State Flow
Max Flow Rate:	7 GPM @ 60psi
Turn-On:	0.3 GPM

400 Captain Neville Drive, Waterbury, CT 06705

info@eemaxinc.com

(800) 543-6163

eemax.com

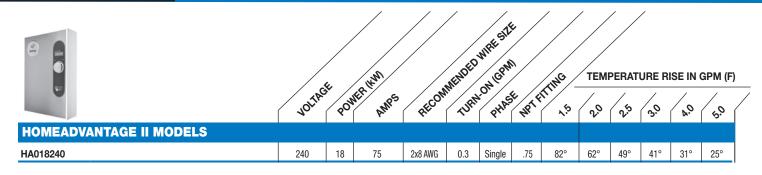


Rolling Acres Cabins 166 John Logsdon Cemetery Rd. Munfordville, KY 42765

RAC-10



NO LEAD



SUGGESTED SPECIFICATION Tankless Water Heater shall be an Eemax,
, withkWkW,vac, and to heat GPM
@ a temperature rise of degrees F.
Heater shall include:
Field-replaceable, non-ferrous, lead free cartridge style element
 Active energy management to ensure optimal application of energy based on real time system demand
Visual user interface
Integrated flow meter
150 PSI pressure rating

	EEMAX SUB	MIT.	TAL		
Engineer/Architect:					
Job Name/Customer:					
Location:					
Contractor:					
Representative:					
HEATER SPECIFICATIONS:	Quantity	kW	Voltage	AMPS	GPM
SpecAdvantage Model #					

*Units with 1 GPM turn-on are specific to 120°F only. Contact Eemax support for applications above 120°F and 1 GPM or less.



166 John Logsdon Cemetery Rd. Munfordville, KY 42765

RAC-10



NO LEAD *

Location: Exterior Wall Header Multi-Loaded Multi-Span Beam

[2015 International Building Code(2015 NDS)]

(3) 1.5 IN x 5.5 IN x 3.5 FT #2 - Spruce-Pine-Fir - Dry Use Section Adequate By: 266.0% Controlling Factor: Moment



StruCalc 9.0 Glenco, Inc. 217 East Main Street Middleburg, PA 17842

of

StruCalc Version 10.0.1.6

3/13/2020 12:59:05 PM

DEFLECTION	<u>s</u> <u>c</u>	<u>Center</u>	
Live Load	0.01	IN L/3087	
Dead Load	0.01	in	
Total Load	0.02	IN L/2141	
Live Load Defl	ection C	riteria: L/240	Total Load Deflection Criteria: L/180

٢	REACTIONS	<u>A</u>		<u>B</u>	
l	Live Load	616	lb	616	lb
l	Dead Load	272	lb	272	lb
l	Total Load	888	lb	888	lb
l	Bearing Length	0.46	in	0.46	in

BEAM DATA	<u>Center</u>	
Span Length	3.5 ft	
Unbraced Length-Top	0 ft	
Unbraced Length-Bottom	3.5 ft	
Live Load Duration Factor	1.15	
Notch Depth	0.00	

MATERIAL PROPERTIES

#2 - Spruce-Pine-Fir

NOTES

•	Base	e Values	Ac	<u>ljusted</u>
Bending Stress:	Fb =	875 psi	Fb' =	1504 psi
	Cd=1.1	5 CF=1.30 C	r=1.15	
Shear Stress:	Fv =	135 psi	Fv' =	155 psi
	Cd=1.1	5		
Modulus of Electicity:		1400 kci	=' -	1400 kci

Modulus of Elasticity: E = 1400 ksi E' = 1400 ksi Comp. $^{\perp}$ to Grain: Fc - $^{\perp}$ = 425 psi Fc - $^{\perp}$ ' = 425 psi

Controlling Moment: 777 ft-lb

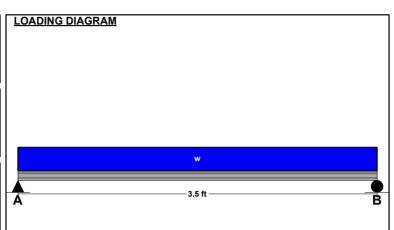
1.75 Ft from left support of span 2 (Center Span)

Created by combining all dead loads and live loads on span(s) 2

Controlling Shear: -657 lb

At a distance d from right support of span 2 (Center Span) Created by combining all dead loads and live loads on span(s) 2

Comparisons with required sections:	Req'd	Provided
Section Modulus:	6.2 in3	22.69 in3
Area (Shear):	6.35 in2	24.75 in2
Moment of Inertia (deflection):	5.24 in4	62.39 in4
Moment:	777 ft-lb	2844 ft-lb
Shear:	-657 lb	2562 lb



UNIFORM LOADS	<u>C</u>	<u>Center</u>
Uniform Live Load	352	plf
Uniform Dead Load	151	plf
Beam Self Weight	5	plf
Total Uniform Load	508	nlf



AGRICULTURE

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03/26/2020

Rolling Acres Cabins 166 John Logsdon Cemetery Rd. Munfordville, KY 42765

RAC-10

Location: Floor Joist

Floor Joist

[2015 International Building Code(2015 NDS)]

1.5 IN x 9.25 IN x 13.5 FT @ 16 O.C. #2 - Spruce-Pine-Fir - Dry Use Section Adequate By: 29.9% Controlling Factor: Moment



Glenco, Inc. 217 East Main Street Middleburg, PA 17842



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DEFLECTIONS	<u>C</u>	<u>enter</u>	
Live Load	0.29	IN L/563	
Dead Load	0.07	in	
Total Load	0.36	IN L/450	
Live Load Defle	ection C	riteria: L/360	Total Load Deflection Criteria: L/240

<u>REACTIONS</u>	<u>A</u>		<u>B</u>	
Live Load	360	lb	360	lb
Dead Load	90	lb	90	lb
Total Load	450	lb	450	lb
Bearing Length	0.71	in	0.71	in

ſ	SUPPORT LOADS	<u>A</u>		<u>B</u>	
	Live Load	270	plf	270	plf
ı	Dead Load	68	plf	68	plf
ı	Total Load	338	plf	338	plf

MATERIAL PROPERTIES

#2 - Spruce-Pine-Fir

	<u>Base</u>	<u>e Values</u>	<u>Ac</u>	<u>ljusted</u>
Bending Stress:	Fb =	875 psi	Fb' =	1107 psi
	Cd=1.0	0 CF=1.10 C	r=1.15	
Shear Stress:	Fv =	135 psi	Fv' =	135 psi
	Cd=1.0	0		
Modulus of Flasticity	F =	1400 ksi	F' =	1400 ksi

Comp. \perp to Grain: Fc - \perp = 425 psi Fc - \perp ' = 425 psi

Controlling Moment: 1519 ft-lb

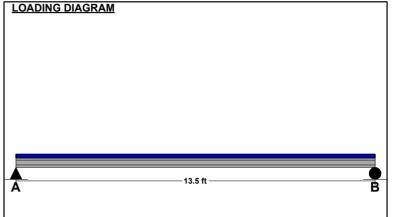
6.75 Ft from left support of span 2 (Center Span) Created by combining all dead loads and live loads on span(s) 2

Controlling Shear: 405 lb

At a distance d from left support of span 2 (Center Span)

Created by combining all dead loads and live loads on span(s) 2

Comparisons with required sections:	Req'd	<u>Provided</u>
Section Modulus:	16.47 in3	21.39 in3
Area (Shear):	4.5 in2	13.88 in2
Moment of Inertia (deflection):	63.26 in4	98.93 in4
Moment:	1519 ft-lb	1973 ft-lb
Shear:	405 lb	1249 lb



JOIST DATA	<u>Cer</u>						
Span Length	13.5	ft					
Unbraced Length-Top	0	ft					
Unbraced Length-Bottom	0	ft					
Floor sheathing applied to top of joists-top of joists fully braced.							
Floor Duration Factor 1.0	0						

JOIST LOADING			
Uniform Floor Loading		Cent	<u>er</u>
Live Load	LL =	40	psf
Dead Load	DL =	10	psf
Total Load	TL =	50	psf
TL Adi. For Joist Spacino	= Tw c	66.7	plf

NOTES





Rolling Acres Cabins 166 John Logsdon Cemetery Rd. Munfordville, KY 42765

RAC-10

Location: H1-1

Multi-Loaded Multi-Span Beam

[2015 International Building Code(2015 NDS)]

(2) 1.5 IN x 9.25 IN x 3.0 FT #2 - Spruce-Pine-Fir - Dry Use Section Adequate By: 64.7% Controlling Factor: Moment



Glenco, Inc. 217 East Main Street Middleburg, PA 17842



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CAUTIONS

* Laminations are to be fully connected to provide uniform transfer of loads to all members

DEFLECTIONS	<u>s</u> c	<u>Center</u>	
Live Load	0.01	IN L/3318	
Dead Load	0.00	in	
Total Load	0.01	IN L/3032	
Live Load Defle	ection C	riteria: L/240	Total Load Deflection Criteria: L/180

REACTIONS	<u>A</u>		<u>B</u>	
Live Load	1744	lb	1744	lb
Dead Load	234	lb	234	lb
Total Load	1978	lb	1978	lb
Bearing Length	1.55	in	1.55	in

BEAM DATA	Center
Span Length	3 ft
Unbraced Length-Top	0 ft
Unbraced Length-Bottom	3 ft
Live Load Duration Factor	1.15
Notch Depth	0.00

MATERIAL PROPERTIES

#2 - Spruce-Pine-Fir

	Base	<u>Values</u>	<u>Adjusted</u>		
Bending Stress:	Fb =	875 psi	Fb' =	1107 psi	
	Cd=1.1	5 CF=1.10			
Shear Stress:	Fv =	135 psi	Fv' =	155 psi	
	Cd=1.1	5			
Madulus of Flacticitus		4400 1:		4400 1:	

Modulus of Elasticity: E = 1400 ksi E' = 1400 ksi Comp. $^{\perp}$ to Grain: Fc - $^{\perp}$ = 425 psi Fc - $^{\perp}$ = 425 psi

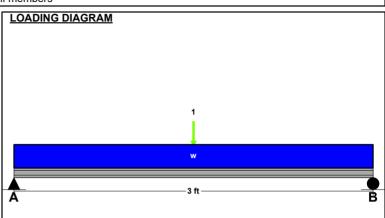
Controlling Moment: 2396 ft-lb 1.5 Ft from left support of span 2 (Center Span)

Created by combining all dead loads and live loads on span(s) 2

Controlling Shear: 1597 lb

At a distance d from left support of span 2 (Center Span) Created by combining all dead loads and live loads on span(s) 2

Comparisons with required sections:	<u>Req'd</u>	<u>Provided</u>
Section Modulus:	25.97 in3	42.78 in3
Area (Shear):	15.43 in2	27.75 in2
Moment of Inertia (deflection):	14.31 in4	197.86 in4
Moment:	2396 ft-lb	3946 ft-lb
Shear:	1597 lb	2872 lb



UNIFORM LOADS		ontor
UNIFURIAL LUADS	<u></u>	<u>enter</u>
Uniform Live Load	352	plf
Uniform Dead Load	151	plf
Beam Self Weight	5	plf
Total Uniform Load	508	plf

POINT LOADS - CENTER SPAN Load Number One Live Load 2432 lb Dead Load 0 lb Location 1.5 ft



NOTES



Rolling Acres Cabins 166 John Logsdon Cemetery Rd. Munfordville, KY 42765

RAC-10

Location: Porch - Floor - P.T. (2) 2x8 SYP #2 Girder

Multi-Loaded Multi-Span Beam

[2015 International Building Code(2015 NDS)] (2) 1.5 IN x 7.25 IN x 6.0 FT Pressure Treated

#2 - Southern Pine - Wet Use Section Adequate By: 101.9% Controlling Factor: Moment



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CAUTIONS

* Laminations are to be fully connected to provide uniform transfer of loads to all members

DEFLECTIONS	<u> </u>	<u>Center</u>	
Live Load	0.04	IN L/1760	
Dead Load	0.01	in	
Total Load	0.05	IN L/1373	
Live Load Defle	ection C	riteria: L/360	Total Load Deflection Criteria: L/240

Γ	REACTIONS	<u>A</u>		<u>B</u>	
ı	Live Load	480	lb	480	lb
ı	Dead Load	136	lb	136	lb
l	Total Load	616	lb	616	lb
	Bearing Length	0.54	in	0.54	in

BEAM DATA	<u>Center</u>
Span Length	6 ft
Unbraced Length-Top	0 ft
Unbraced Length-Bottom	6 ft
Live Load Duration Factor	1.00
Notch Depth	0.00

MATERIAL PROPERTIES

#2 - Southern Pine

	<u>Base</u>	<u>Values</u>	<u>Adjusted</u>			
Bending Stress:	Fb =	925 psi	Fb' =	851 psi		
	0-1-4-00	05-4000	-4 45 00	0.00		

 $Cd=1.00 \ CF=1.00 \ Cr=1.15 \ Ci=0.80$ Shear Stress: Fv = 175 psi Fv' = 136 psi

Cd=1.00 Cm=0.97 Ci=0.80

Modulus of Elasticity: E = 1400 ksi E' = 1197 ksi

Cm=0.90 Ci=0.95

Comp. \perp to Grain: Fc - \perp = 565 psi Fc - \perp ' = 379 psi

Cm=0.67

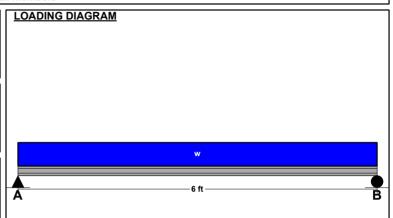
Controlling Moment: 923 ft-lb 3.0 Ft from left support of span 2 (Center Span)

Created by combining all dead loads and live loads on span(s) 2

Controlling Shear: -492 lb

At a distance d from right support of span 2 (Center Span) Created by combining all dead loads and live loads on span(s) 2

Comparisons with required sections:	Req'd	Provided
Section Modulus:	13.02 in3	26.28 in3
Area (Shear):	5.44 in2	21.75 in2
Moment of Inertia (deflection):	19.49 in4	95.27 in4
Moment:	923 ft-lb	1864 ft-lb
Shear:	-492 lb	1969 lb



UNIFORM LOADS	<u>C</u>	<u>Center</u>
Uniform Live Load	160	plf
Uniform Dead Load	40	plf
Beam Self Weight	5	plf
Total Uniform Load	205	plf



NOTES



Rolling Acres Cabins 166 John Logsdon Cemetery Rd. Munfordville, KY 42765

RAC-10

Location: Porch - Floor - P.T. 2x6 Floor Joist

Multi-Loaded Multi-Span Beam

[2015 International Building Code(2015 NDS)]

1.5 IN x 7.25 IN x 8.0 FT Pressure Treated

#2 - Southern Pine - Wet Use Section Adequate By: 46.6% Controlling Factor: Moment



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DEFLECTIONS	<u> </u>	<u>enter</u>	
Live Load	0.09	IN L/1117	
Dead Load	0.03	in	
Total Load	0.11	IN L/860	
Live Load Defle	ction C	riteria: L/360	Total Load Deflection Criteria: L/240

REACTIONS	<u>A</u>		<u>B</u>	
Live Load	213	lb	213	lb
Dead Load	64	lb	64	lb
Total Load	277	lb	277	lb
Bearing Length	0.49	in	0.49	in

BEAM DATA	Се	enter
Span Length	8	ft
Unbraced Length-Top	0	ft
Unbraced Length-Bottom	8	ft
Live Load Duration Factor	1	.00
Notch Depth	0	.00

MATERIAL PROPERTIES

#2 - Southern Pine

Shear Stress: Fv = 175 psi Fv' = 136 ps Cd=1.00 Cm=0.97 Ci=0.80

Modulus of Elasticity: E = 1400 ksi E' = 1197 ksi Cm=0.90 Ci=0.95

Comp. \perp to Grain: Fc - \perp = 565 psi Fc - \perp ' = 379 psi

Cm=0.67

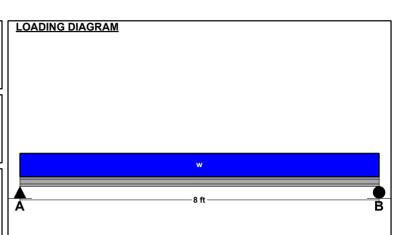
Controlling Moment: 553 ft-lb 4.0 Ft from left support of span 2 (Center Span)

Created by combining all dead loads and live loads on span(s) 2

Controlling Shear: -238 lb

At a distance d from right support of span 2 (Center Span) Created by combining all dead loads and live loads on span(s) 2

Comparisons with required sections:	Req'd	<u>Provided</u>
Section Modulus:	8.96 in3	13.14 in3
Area (Shear):	2.63 in2	10.88 in2
Moment of Inertia (deflection):	15.36 in4	47.63 in4
Moment:	553 ft-lb	810 ft-lb
Shear:	-238 lb	985 lb



UNIFORM LOADS	<u>C</u>	<u>Center</u>
Uniform Live Load	53	plf
Uniform Dead Load	13	plf
Beam Self Weight	3	plf
Total Uniform Load	69	plf



NOTES



Rolling Acres Cabins 166 John Logsdon Cemetery Rd. Munfordville, KY 42765

RAC-10

Location: Porch - Roof - 2x6 T&G Roof Deck

Multi-Loaded Multi-Span Beam

[2015 International Building Code(2015 NDS)]

1.5 IN x 5.5 IN x 2.0 FT (Weak Axis Bending - Flat Use)

#2 - Spruce-Pine-Fir - Dry Use Section Adequate By: 3029.6% Controlling Factor: Moment



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DEFLECTION	S Center		
Live Load	0.00 IN L/M	AX	
Dead Load	0.00 in		
Total Load	0.00 IN L/75	599	
Live Load Def	ection Criteria: L	_/240	Total Load Deflection Criteria: L/180
REACTIONS	A	В	

Γ	REACTIONS	<u>A</u>		<u>B</u>	
l	Live Load	10	lb	10	lb
l	Dead Load	9	lb	9	lb
l	Total Load	19	lb	19	lb
	Bearing Length	0.01	in	0.01	in

BEAM DATA	Center
Span Length	2 ft
Unbraced Length-Top	0 ft
Unbraced Length-Bottom	2 ft
Live Load Duration Factor	1.15
Notch Depth	0.00

MATERIAL PROPERTIES

#2 - Spruce-Pine-Fir

	Base '	<u>Values</u>	<u>Adjı</u>	<u>ısted</u>
Bending Stress:	Fb =	875 psi	Fb' =	1730 psi
	Cd=1.15	CF=1.30 Cr	≃1.15 Cfu	=1.15
Shear Stress:	Fv =	135 psi	Fv' =	155 psi
	Cd=1.15			
Modulus of Elasticity:	E =	1400 ksi	E' =	1400 ksi
Comp. [⊥] to Grain:	Fc - ⊥ =	425 psi	Fc - 上 =	425 psi

Controlling Moment: 10 ft-lb

1.0 Ft from left support of span 2 (Center Span)

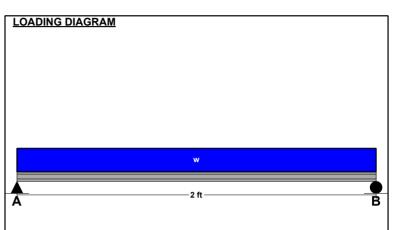
Created by combining all dead loads and live loads on span(s) 2

Controlling Shear: 17 lb

At a distance d from left support of span 2 (Center Span)

Created by combining all dead loads and live loads on span(s) 2

Comparisons with required sections:	<u>Req'd</u>	<u>Provided</u>
Section Modulus:	0.07 in3	2.06 in3
Area (Shear):	0.16 in2	8.25 in2
Moment of Inertia (deflection):	0.04 in4	1.55 in4
Moment:	10 ft-lb	297 ft-lb
Shear:	17 lb	854 lb



UNIFORM LOADS	(Center
Uniform Live Load	10	plf
Uniform Dead Load	8	plf
Beam Self Weight	2	plf
Total Uniform Load	19	plf



NOTES

Thickness increased for stacking with 23/32" OSB.



Rolling Acres Cabins 166 John Logsdon Cemetery Rd. Munfordville, KY 42765

RAC-10

Location: Porch - Roof - P.T. 4x6 Rafter

Multi-Loaded Multi-Span Beam

[2015 International Building Code(2015 NDS)]

3.5 IN x 5.5 IN x 7.33 FT (Actual 7.4 FT)

Select Structural - Eastern White Pine - Dry Use

Section Adequate By: 492.2% Controlling Factor: Deflection



Glenco, Inc. 217 East Main Street Middleburg, PA 17842



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DEFLECTION	<u>S</u> <u>Center</u>	
Live Load	0.05 IN L/1954	
Dead Load	0.04 in	
Total Load	0.08 IN L/1066	
Live Load Defle	ection Criteria: L/240	Total Load Deflection Criteria: L/180

REACTIONS	<u>A</u>		<u>B</u>	
Live Load	147	lb	147	lb
Dead Load	122	lb	122	lb
Total Load	269	lb	269	lb
Bearing Length	0.22	in	0.22	in

BEAM DATA	<u>Ce</u>	<u>nter</u>		
Span Length	7.33	ft		
Unbraced Length-Top	0	ft		
Unbraced Length-Bottom	7.33	ft		
Beam End Elevation Difference	ence	1	ft	
Live Load Duration Factor		1.15		
Notch Depth		0.00		

MATERIAL PROPERTIES

Select Structural - Eastern White Pine

Bending Stress: $\frac{\text{Base Values}}{\text{Fb} = 1250 \text{ psi}}$ $\frac{\text{Adjusted}}{\text{Fb}' = 2149 \text{ psi}}$

Cd=1.15 CF=1.30 Cr=1.15

Shear Stress: Fv = 135 psi Fv' = 155 psi

Cd=1.15

Modulus of Elasticity: E = 1200 ksi E' = 1200 ksi Comp. $^{\perp}$ to Grain: Fc - $^{\perp}$ = 350 psi Fc - $^{\perp}$ ' = 350 psi

Controlling Moment: 492 ft-lb

3.666 Ft from left support of span 2 (Center Span)

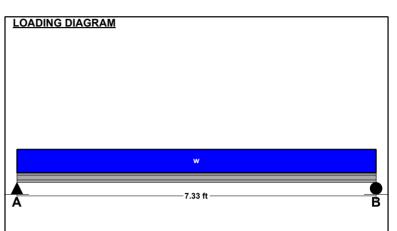
Created by combining all dead loads and live loads on span(s) 2

Controlling Shear: 234 lb

At a distance d from left support of span 2 (Center Span)

Created by combining all dead loads and live loads on span(s) 2

Comparisons with required sections:	Req'd	<u>Provided</u>
Section Modulus:	2.75 in3	17.65 in3
Area (Shear):	2.26 in2	19.25 in2
Moment of Inertia (deflection):	8.19 in4	48.53 in4
Moment:	492 ft-lb	3160 ft-lb
Shear:	234 lb	1992 lb



UNIFORM LOADS	C	<u>enter</u>
Uniform Live Load	40	plf
Uniform Dead Load	30	plf
Beam Self Weight	3	plf
Total Uniform Load	73	plf



NOTES



Rolling Acres Cabins 166 John Logsdon Cemetery Rd. Munfordville, KY 42765

RAC-10

Location: Porch Header Multi-Loaded Multi-Span Beam

[2015 International Building Code(2015 NDS)]

3.5 IN x 5.5 IN x 6.0 FT #2 - Southern Pine - Wet Use Section Adequate By: 109.2% Controlling Factor: Moment



Glenco, Inc. 217 East Main Street Middleburg, PA 17842



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1	DEFLECTION	<u>s</u> (<u>Center</u>	
ı	Live Load	0.05	IN L/1510	
ı	Dead Load	0.04	in	
ı	Total Load	0.09	IN L/841	
ı	Live Load Defl	ection C	riteria: L/240	Total Load Deflection Criteria: L/180

_					
	REACTIONS	<u>A</u>		<u>B</u>	
	Live Load	300	lb	300	lb
	Dead Load	239	lb	239	lb
١.	Total Load	539	lb	539	lb
	Bearing Length	0.41	in	0.41	in

BEAM DATA	Center
Span Length	6 ft
Unbraced Length-Top	0 ft
Unbraced Length-Bottom	6 ft
Live Load Duration Factor	1.15
Notch Depth	0.00

MATERIAL PROPERTIES

#2 - Southern Pine

	Base	e Values	<u>Adjusted</u>		
Bending Stress:	Fb =	1000 psi	Fb' =	1150 psi	
	Cd=1.1	5 CF=1.00			
Shear Stress:	Fv =	175 psi	Fv' =	195 psi	
	Cd=1.1	5 Cm=0.97			
Modulus of Elasticity:	E =	1400 ksi	E' =	1260 ksi	
	Cm=0.	90			
Comp. [⊥] to Grain:	Fc-⊥:	= 565 psi	Fc - 上' =	379 psi	
	Cm=0.	67			

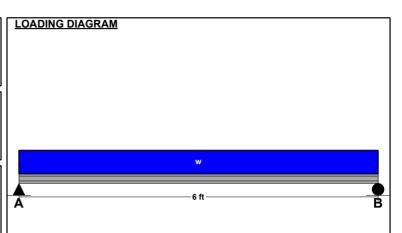
Controlling Moment: 808 ft-lb 3.0 Ft from left support of span 2 (Center Span)

Created by combining all dead loads and live loads on span(s) 2

Controlling Shear: -463 lb

At a distance d from right support of span 2 (Center Span) Created by combining all dead loads and live loads on span(s) 2

Comparisons with required sections:	Req'd	<u>Provided</u>
Section Modulus:	8.43 in3	17.65 in3
Area (Shear):	3.56 in2	19.25 in2
Moment of Inertia (deflection):	10.39 in4	48.53 in4
Moment:	808 ft-lb	1691 ft-lb
Shear:	-463 lb	2505 lb



UNIFORM LOADS	<u>C</u>	<u>enter</u>
Uniform Live Load	100	plf
Uniform Dead Load	75	plf
Beam Self Weight	5	plf
Total Uniform Load	180	plf



NOTES



Rolling Acres Cabins 166 John Logsdon Cemetery Rd. Munfordville, KY 42765

RAC-10



RESIDENTIAL LIGHT GAUGE **APPLICATION GUIDE**

Lebanon, PA • 888.339.0059 Bridgton, ME • 888.339.0059 Howe, IN • 866.562.3782 www.everlastroofing.com





239 North 5th Avenue Lebanon, PA 17046 Office: 888-339-0059 Fax: 717-270-6569

P.O. Box 278, 81 South High St. Bridgton, ME 04009-0278 Office: 888-339-0059

Fax: 207-647-5599

7180 N 050 E Howe, IN 46746 Office: 866-562-3782 Fax: 260-562-3738



Residential Light Guage

Residential Trim

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Application Guidelines									
Safety and Minimum Slope	LG 04								
Handling and Storage	LG 05								
Installation / Ventilation / Cutting	LG 06								

Residential Details

Coverage / Overlap/Screw / Nail Patterns

-	Active that Details	
	Residential Ridge and Hip Details	LG 09 - LG 12
	Residential Endwall Details	LG 13 - LG 15
	Residential Sidewall Details	LG 16 - LG 17
	Residential Rake & Gable Details	LG 18 - LG 20
	Residential Formed Valley Detail	LG 21
	Residential Gambrel Detail	LG 22
	Residential Drip Edge Detail	LG 23
	Residential Fascia & Soffit Detail	LG 24
	Residential Pipe Boot Detail	LG 25
	Residential Chimney Detail	LG 26 - 27

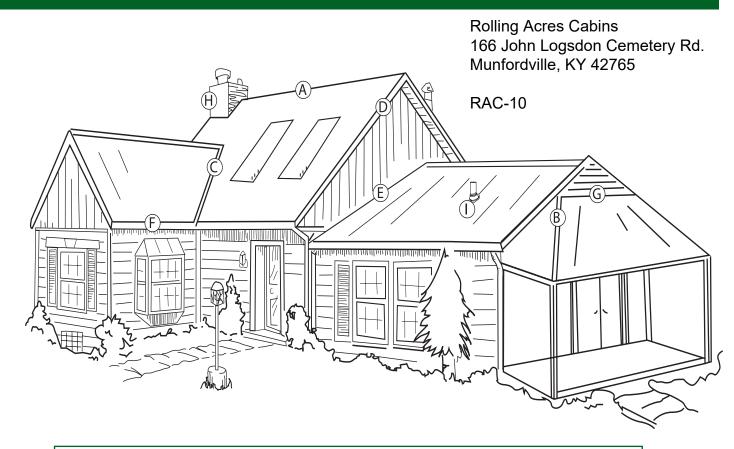


LG 03

LG 07 - LG 08



Residential Light Guage



Ridge Eave / Drip Edge Hip **Endwall** Chimney **Valley** Gable **Pipe Flashing**

Sidewall

Purpose:

This application guide is intended to assist in the application of light gauge Everlast Roofing products on residential structures. The details and illustrations in this manual may not be applicable to all building plans or field situations. It is the buyer's responsibility to verify all applicable code requirements, check all field measurements, and determine suitability of the material for the job.



Residential Light Guage

Application Guidelines

Safety:

Always work safely when installing metal products. Use extreme caution on a roof at all times, and wear gloves and safety glasses to avoid injury. Hearing protection should be used when power-cutting metal panels. Do not walk on panels until all fasteners are installed. Metal panels are slippery when wet, dusty, frosty or oily. Do not attempt to walk on a metal roof under these conditions. Always use OSHA recommended safety harness or equipment when working on a roof. Wear soft-soled shoes to improve traction and to minimize damage to the paint finish. Always be aware of your position on the roof relative to any roof openings, roof edges, co-workers, and penetrations. Installing metal panels on a windy day can be dangerous and should be avoided. Consult OSHA guidelines for more comprehensive safety requirements.

Minimum Slope:

Everlast Roofing light gauge products are designed to be installed on pitches of no less than 3:12. Please contact an Everlast sales representative for product recommendations on lower sloped roofs.

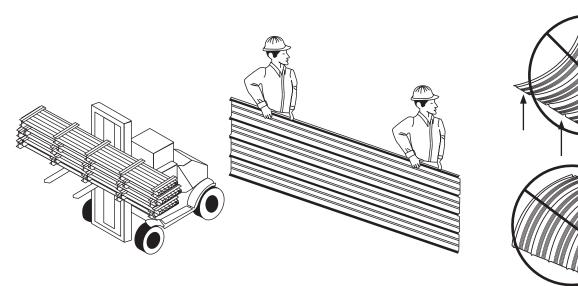




Residential Light Guage

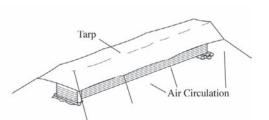
Handling:

- Do not lift panels from ends while flat. Lift the panels on edge when moving individal panels or when moving panels onto the roof.
- Do not unload in a jerking or bouncing fashion. While unloading, the bundle must be handled at lift point, specified by Everlast Roofing, Inc. Panels greater than 25' long should be unloaded using spreader bar to prevent panels from bending.
- Although the paint coating is tough, dragging panels across the surface of one another will almost certainly damage the finish. Improper handling of metal panels may cause scratches to the paint finish. ELR offers matching touch up paint in a variety of standard colors in the event of any scratches to the paint finish. Please note: touch up paint will not weather as well or at the same rate as the original coating or finish.



Storage:

If the material is not to be used immediately, it should be stored in a dry place. Moisture trapped between sheets may cause damage to the paint system. The paint system may become soft or water stains may appear which can detract from the appearance and affect the service life of the material. To avoid problems store the materials in a well-ventilated dry area. Stack the materials in an incline position. DO NOT USE PLASTIC TO COVER MATERIALS. THIS CAN CAUSE SWEATING OR CONDENSATION!



Rolling Acres Cabins 166 John Logsdon Cemetery Rd. Munfordville, KY 42765

RAC-10



Everlast Roofing Inc. Light Gauge Manual **Roofing, Inc.**

Residential Light Guage

Installation Recommendations:

- Use 30 lbs.. Felt paper and/or ice and water shield as an underlayment
- Panels should be installed against any prevailing wind
- Panels should be installed square, plumb and properly aligned to one another.
- Galvanized and galvalume panels should remain void of any condensation containing copper, lead or uncoated steel materials
- In situations where an endlap of a panel is required, be certain to overlap upper panel a minimum of 8" over lower panel and apply sealant and butyl lap tape uniformly between the two panels.

Ventilation/Insulation:

Proper design and installation of vapor barriers and ventilation systems are important to prevent condensation and the resulting problems of moisture damage and loss of insulation efficiency. Condensation occurs when moistureladen air comes in contact with a surface temperature equal to or below the dew point of the air. This phenomenon creates problems that are not unique to metal buildings; these problems are common to all types of construction. In addition to providing resistance to heat transfer, insulation can also protect against condensation forming on cold surfaces, either inside the building or within the wall/roof/system cavity. The arrangement of the building's insulation system and vapor retarder is the responsibility of the building designer. These are some basic guides to help control condensation.

- The insulation should have a vapor retarder face on the "warm" side of the insulation. For most buildings, this means that the vapor retarder is on the inside surface (toward the building's interior).
- B. The thickness of the insulation must be designed to maintain temperature of the vapor retarder above the interior dew point, using the worst-case expected outside temperature.
- C. All perimeter conditions, seams, and penetrations of the vapor retarder must be adequately sealed in order to provide a continuous membrane to resist the passage of water vapor.
- D. Building ventilation, whether by gravity ridge vent, power-operated fans, or other means, contributes significantly to reduced condensation. The movement of air to the outside of the building reduces the interior level of vapor pressure. On the buildings that have an attic space or are being retrofitted with metal roofing systems, vents should be placed at both the eave and peak of the roof in order to prevent a buildup of moisture (humidity) in the attic space. Contact your local building code official or an engineer on proper ventilation practices for you area.

Cutting:

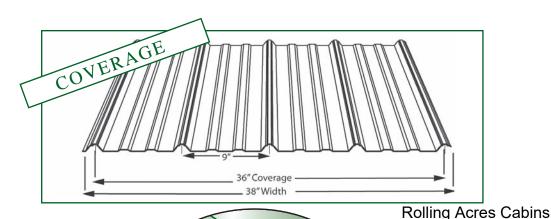
LG-6

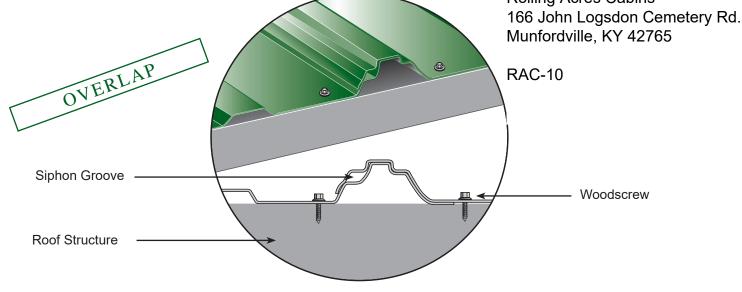
Everlast recommends the use of tin snips or a "nibbler" type electric tool to field cutting metal panels. Cutting metal panels may create metal shavings.

- These shavings and/or chips must be removed immediately from the panel surface. Failure to remove such shavings or chips may cause staining and/or rust on the panel.. Any such surface damage will void
- В. For your protection goggles should be worn when cutting metal panels and flashing.

Coverage/Overlap and Screw/Nail Pattern of Everlast IITM







Over-driven

SCREW PATTERN

Everlast II

NAIL PATTERN



Correct

Under-driven

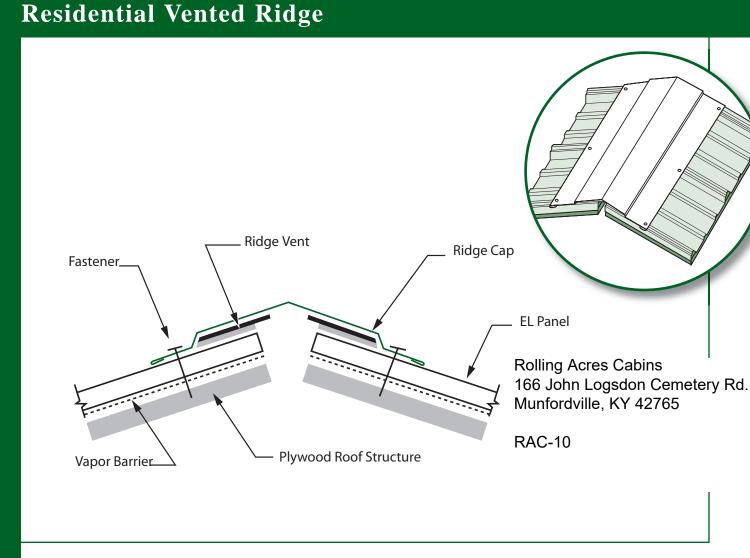
*Note - minimum of 1" fastener penetration is recommended

^{*} This same principle applies when driving steel fasteners (see pages LG07 & LG08)

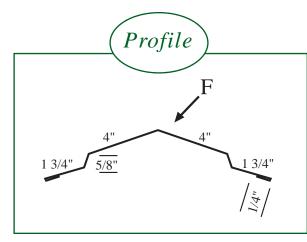


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Coverage/Overlap and Screw/Nail Pattern of EverdrainTM **APPROVED** DATE 4/1/20 PFS CORPORATION OVERLAP Cottage Grove, WI Siphon Groove Wood Screw Roof Structure NAIL PATTERN SCREW PATTERN Everdrain



01A Universal Ridge Cap TLURC1



Over-driven

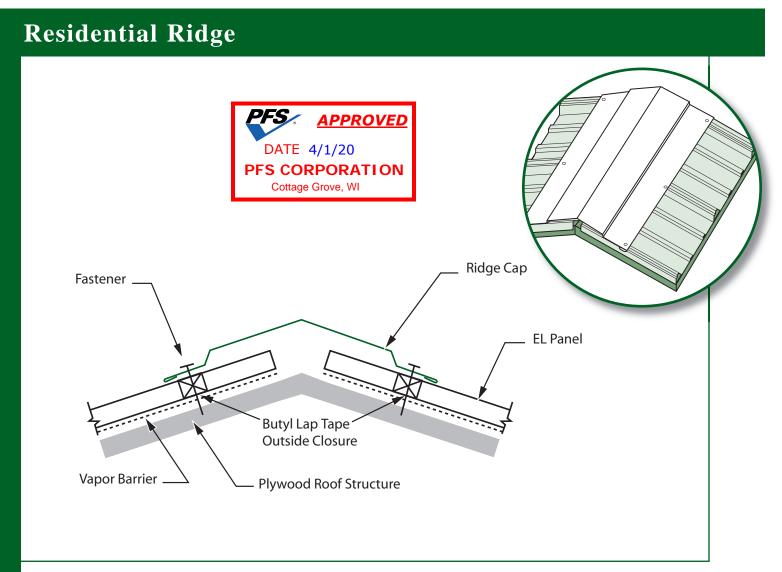
Correct

Under-driven

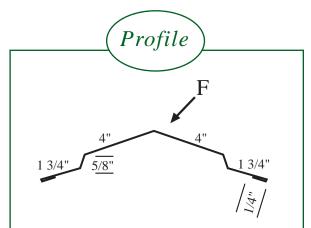


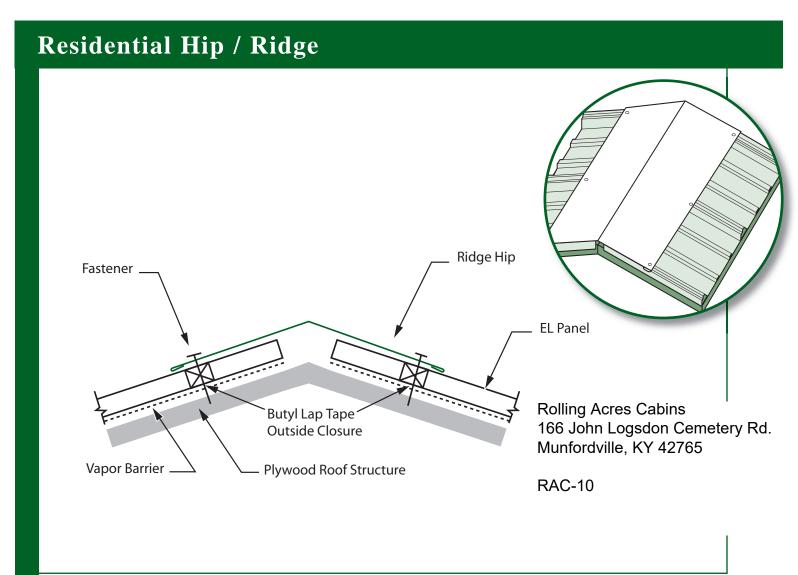
Everlast Roofing, Inc.

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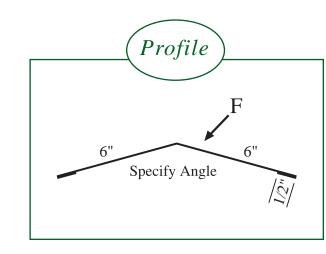


01A Residential Ridge Cap TLURC1





01A Universal Ridge Cap TLURC1

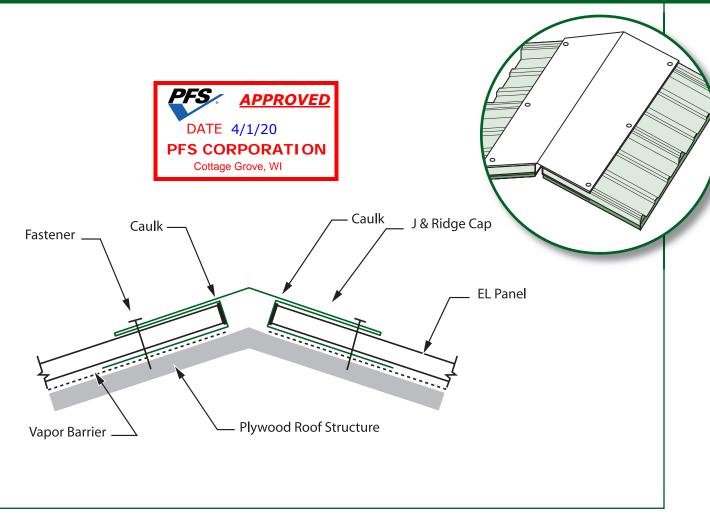




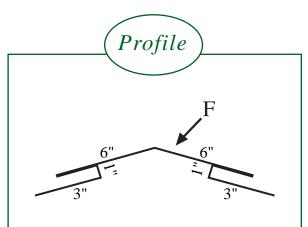
Everlast Roofing, Inc.

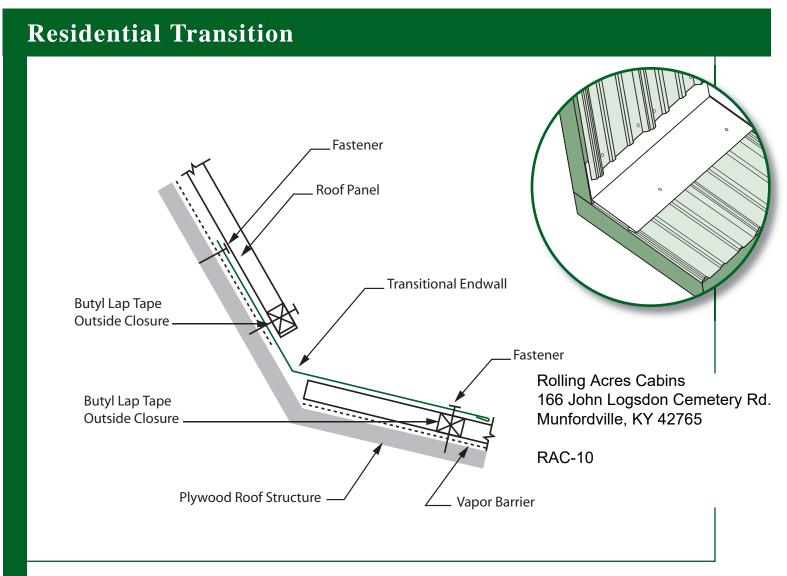
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Residential J & Ridge Residential Light Guage

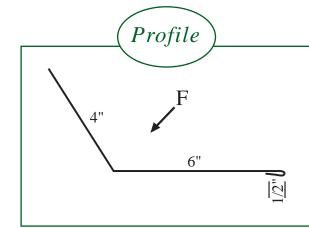


89 J and Ridge Cap Trim TLJRT





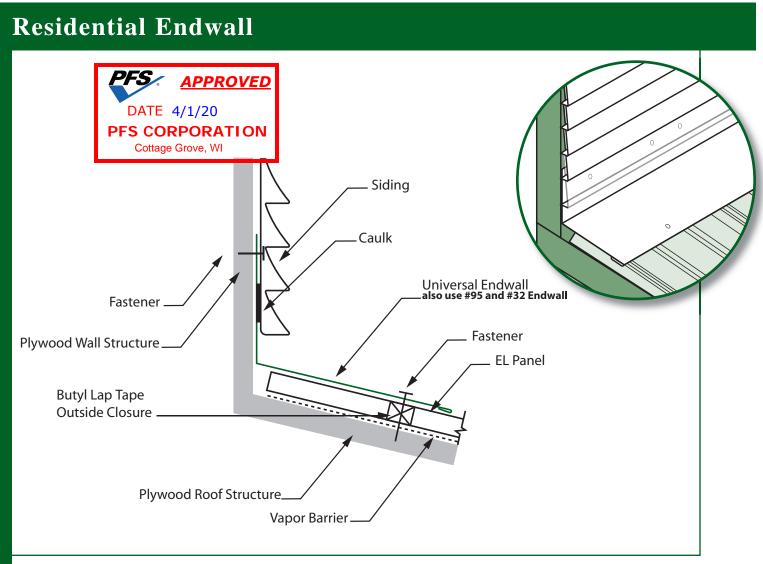
11 Universal Endwall TLUE11



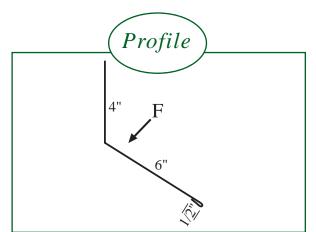


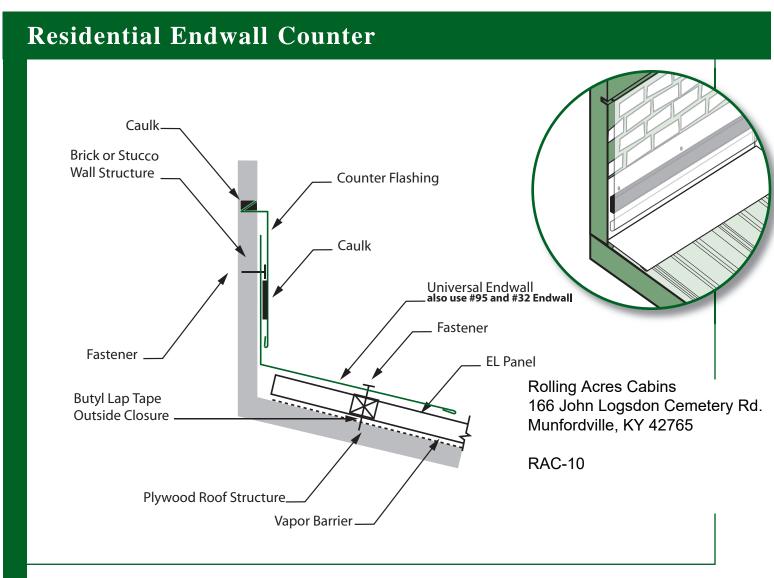
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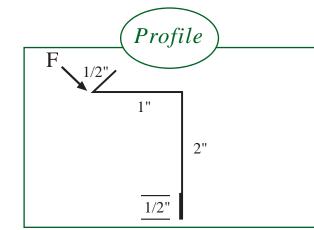


Universal Endwall TLUE11





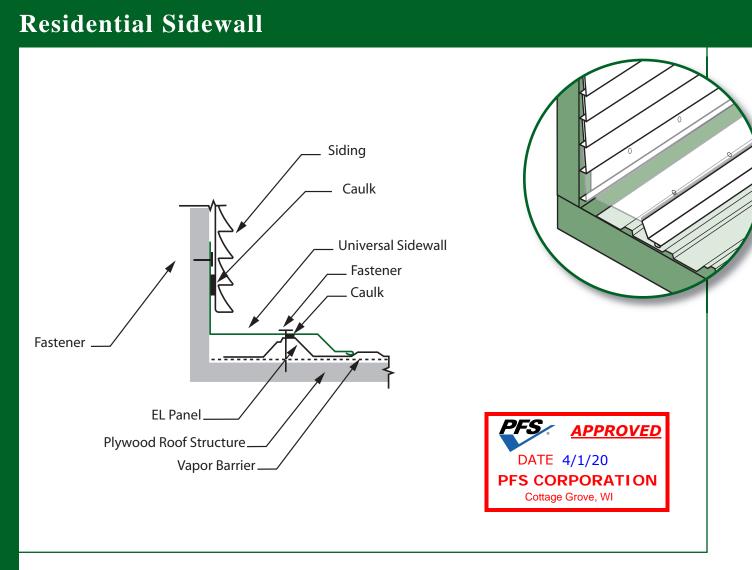
Counter Flashing 29CF122



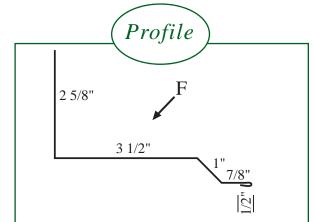


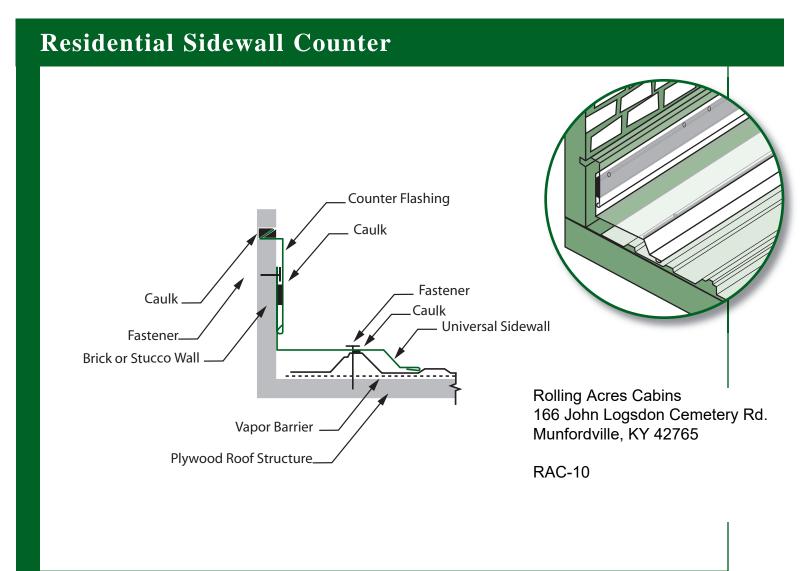
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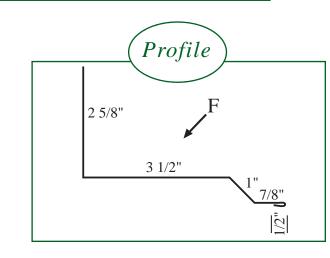


08 Universal Sidewall TLUS8





08 Universal Sidewall TLUS8



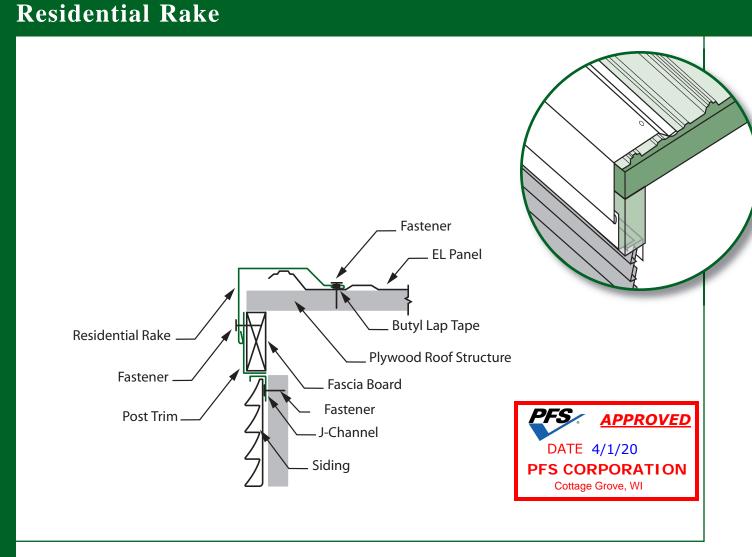


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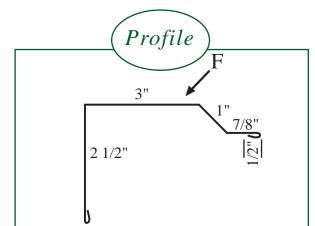
Light Gauge Manual

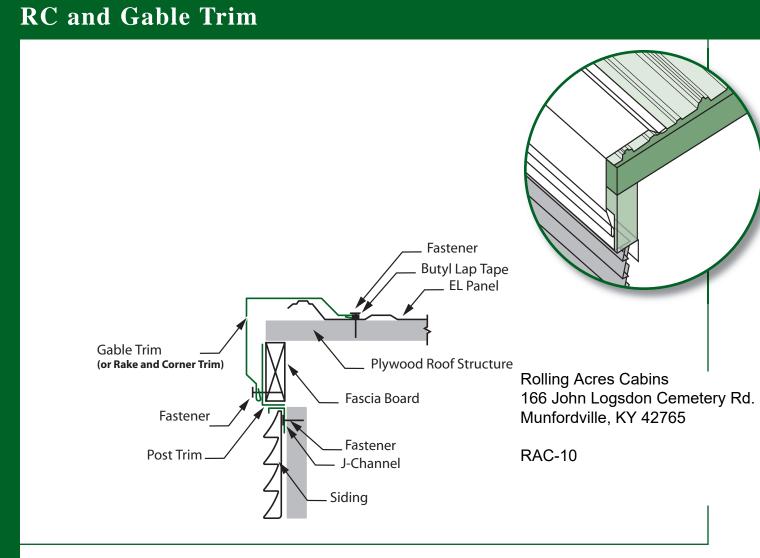


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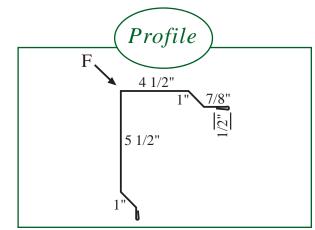


18 Residential Rake TLRR18





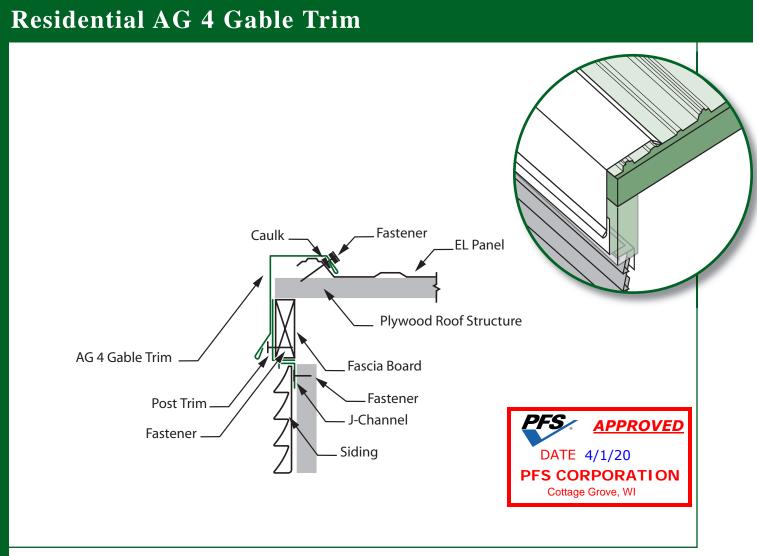
05 Gable Trim TLGT5



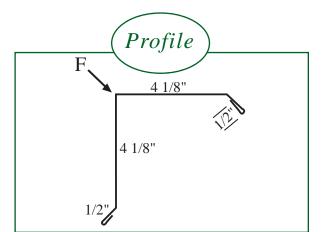


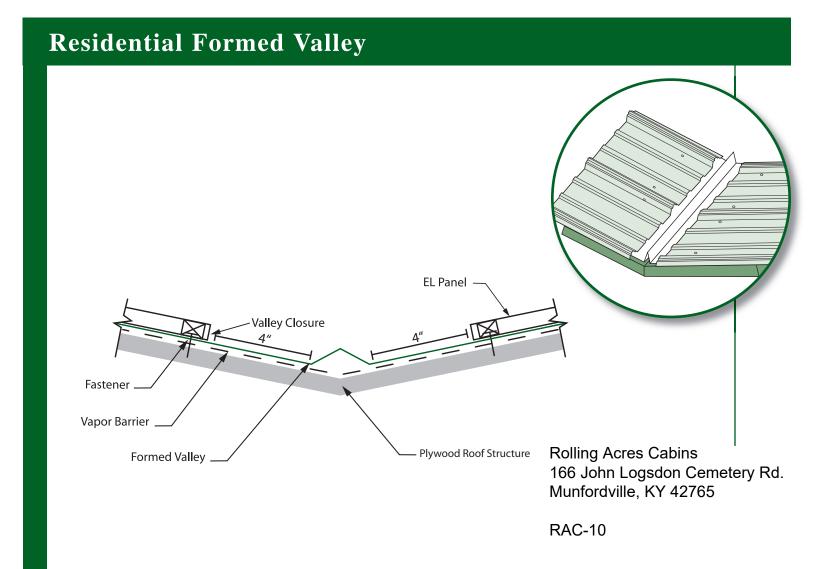
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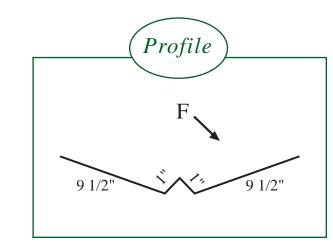


91 AG 4 Gable Trim TLAG4





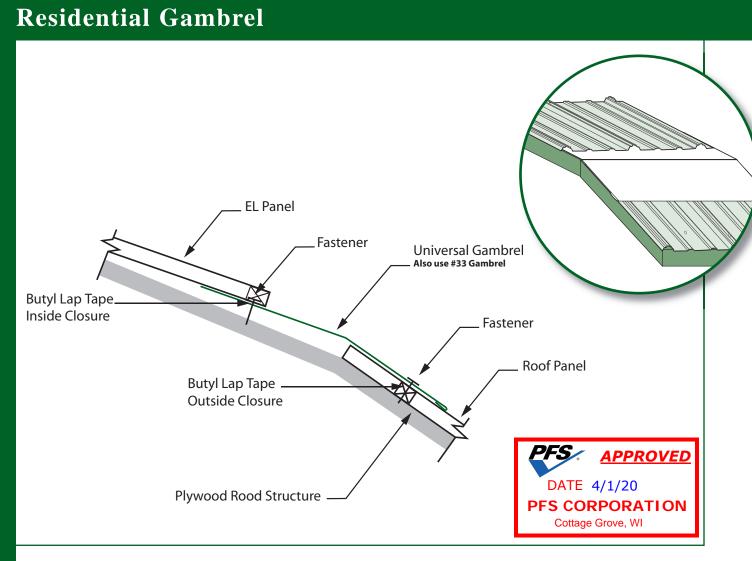
19 Formed Valley TLFV19



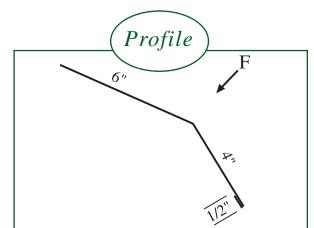


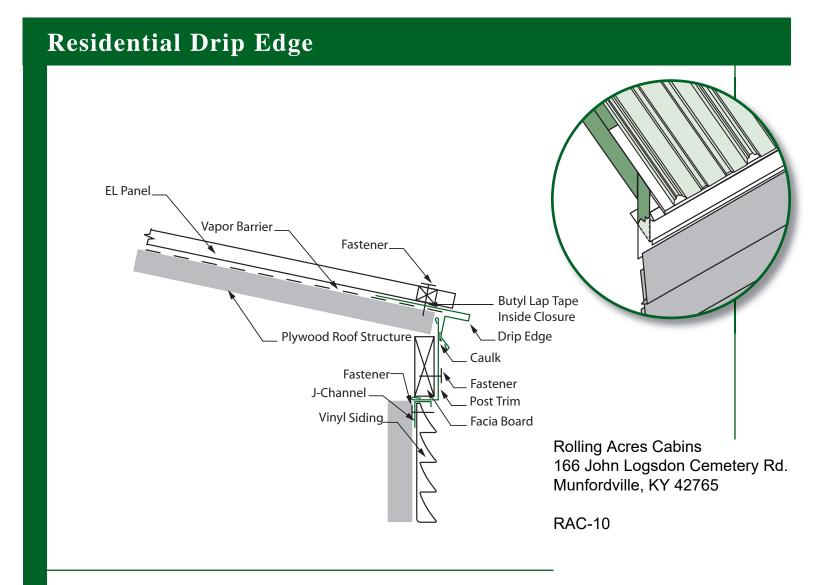
Everlast Roofing, Inc.

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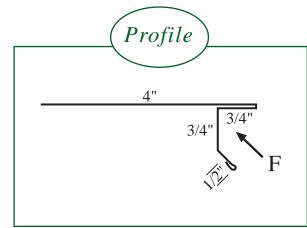


28 Universal Gambrel TLGT28





60 Drip Edge TL6.5F60





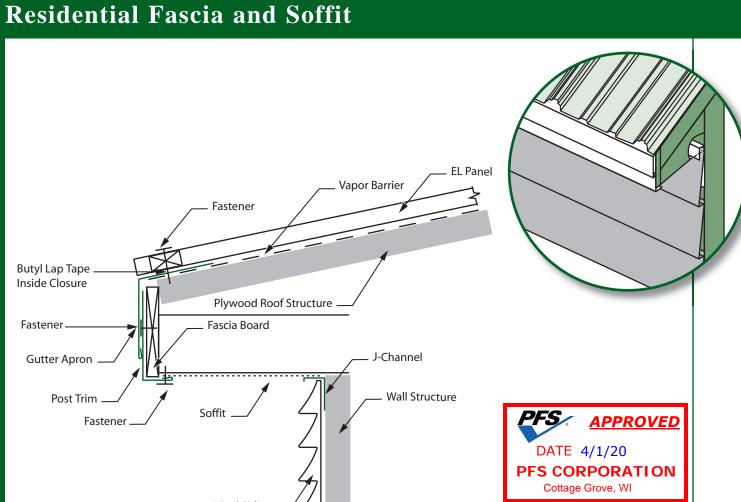
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Residential Pipe Boot Detail

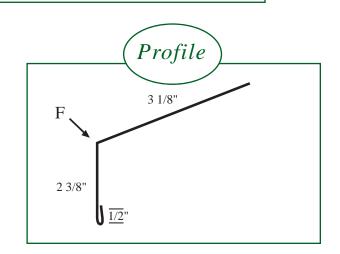
Rolling Acres Cabins 166 John Logsdon Cemetery Rd. Munfordville, KY 42765

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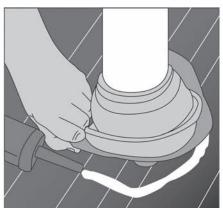
Pipe Boot Installation

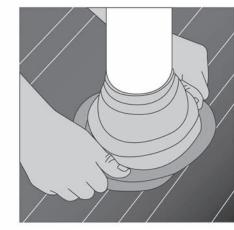


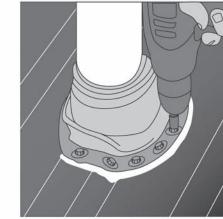
29 Gutter Apron TLGA29

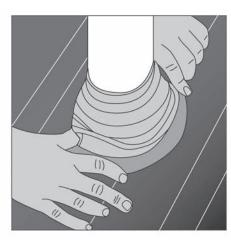


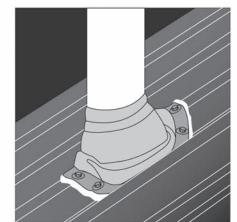








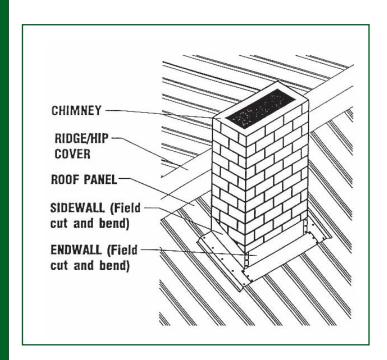


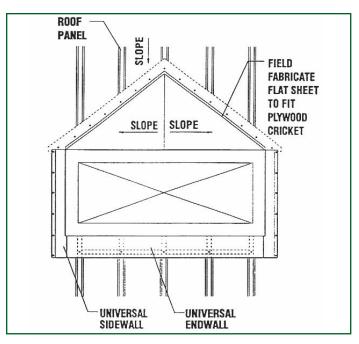


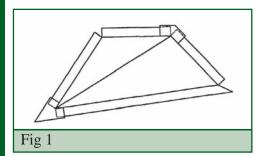


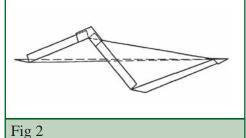
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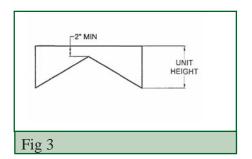
Residential Chimney Detail

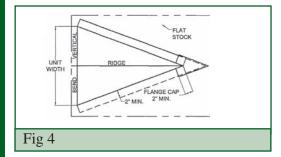


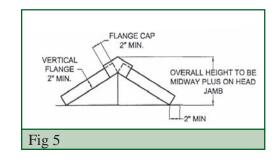












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Rolling Acres Cabins 166 John Logsdon Cemetery Rd. Munfordville, KY 42765



RAC-10

12300 Ford Rd, Suite 110 Dallas, Texas 75234

eaglemetal.com

The truss designs referenced below have been prepared by me or under my direct supervision based on the truss design criteria and requirements ("design criteria") provided by **Precision Truss & Metal**.

These truss designs are intended for the fabrication of individual building components that will perform to the design criteria provided. Any variance from the design criteria will render the affected truss designs inapplicable.

Listed below are the truss designs included in this package and covered by this seal.

Job: Rolling Acres (Display #3) - 1123362 G1, G2, G3, G4, GE1, S1, S2, S3, T1, T3, T4, T5, T6

Any location identification is for file reference only. No determination of the appropriateness of design criteria for any specific project has been made in preparing the truss designs.

Please refer to individual truss designs for specific design criteria.



Arturo A. Hernandez (TN, 110350)

My license renewal date for the state of TN is 11/30/2021.

IMPORTANT NOTE: The responsibility of the engineer sealing this package, as a Truss Engineer, is solely for design of individual trusses as individual building components based upon design criteria provided by others and set forth in the referenced truss drawings. The truss design criteria for the components have not been verified as appropriate for any particular building, project or use. Adequacy and suitability of design criteria and requirements for the truss designs for any specific project are the responsibility of the building designer, not the Truss Engineer, per ANSI/TPI-1, Chapter 2.



¬EAGLE METAL

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MARNING: Failure to follow may result in property damage or personal injury.

DESIGN NOTES

RAC-10

- The Truss Design Drawing(s) provided with these General Notes have been prepared under and are subject to ANSI/TPI1.
 Capitalized terms have the meanings provided in ANSI/TPI1.
- Copies of each Truss Design Drawing shall be furnished to the installation contractor, Building Designer, Owner and all persons fabricating, handling, installing, bracing, or erecting the trusses.

DESIGN LIMITATIONS

- 3. The Truss Design Drawing is based upon specifications provided by the Building Designer in accordance with ANS1/TPI1. Neither the Truss Designer, Eagle, nor an engineer who seals this design (if any) assumes any responsibility for the adequacy or accuracy of specifications provided by the Building Designer.
- 4. The Building Designer is solely responsible for the suitability based upon the Truss Design Drawing and shall be responsible for reviewing and verifying that the information shown is in general conformance with the design of the Building.
- 5. Each Truss Design Drawing is for the individual building component (a truss). A seal on the Truss Design Drawing indicates acceptance of professional engineering responsibility solely for the individual truss.
- Each Truss Design Drawing assumes trusses will be suitably protected from the environment.

HANDLING, INSTALLING, & BRACING

- 7. Refer to BCSI for handling, installing, restraining and bracing trusses. Copies can be obtained from the Truss Plate Institute (TPI), 218 N Lee Street, Suite 312, Alexandria, VA 22314, www.tpinst.org or SBCA, 6300 Enterprise Lane, Madison, WI 53719, www.sbcindustry.com.
- 8. Bracing shown on each Truss Design Drawing is for lateral support of individual truss components only to reduce buckling lengths. All temporary and permanent bracing, including lateral load and diagonal or cross bracing, are the responsibility, respectively, of the erector and Building Designer.
- Eagle is not responsible for improper truss fabrication, handling, erection or bracing.
- Compression chords shall be laterally braced by the roof or floor sheathing, directly attached, or have purlins provided at spacing shown, unless noted otherwise.



- Bottom chord required bracing shall be at 10 ft spacing or less, if no structural rated ceiling is installed, unless noted of erwise.
 Cottage Grove, WI
 Strongbacking thall be installed on all parallel chord trusses.
- Strongbacking thall be installed on all parallel chord to issue including flooring systems, to limit deflection and reduce vibration. Refer to BCSI-B7.
- Never exceed the design loading shown and never stack building or other materials on inadequately braced truss; refer to BCSI.
- Concentration of construction loads greater than the design loads shall not be applied to the trusses at any time; refer to BCSI.
- 15. Trusses shall be handled with care prior to erection to avoid damage. Refer to BCSI for recommended truss handling and erection.

MATERIALS & FABRICATION

- **16.** Lumber moisture content shall be 19% or less at the time of fabrication unless noted otherwise.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- **18.** Unless expressly noted, the truss designs are not applicable for use with fire retardant or preservative treated lumber.
- Plates shall be applied on both faces of truss at each joint and embedded fully. Knots and wane at joint locations shall be regulated in accordance with ANSI/TPI1.
- For a specified plate gauge and grade, the specified size is a minimum.
- **21.** Connections not shown are the responsibility of others.
- Adequate support shall be provided to resist gravity, lateral, uplift loads.
- **23.** For 4X2 truss orientation, locate plates 0 1/16" from outside the edge of the truss.
- **24.** Fabrication of truss shall be in accordance with ANSI/TPI1.

OTHER NOTES

- **25.** Camber is a non-structural consideration and is the responsibility of truss fabricator.
- **26.** Do not cut or alter any truss member or plate without prior approval from a professional engineer.
- **27.** Lumber design values are in accordance with ANSI/TPI; lumber design values are by others.
- 28. Install specified hangers per manufacturer recommendations.

SYMBOLS

PLATE SIZE

- **3X4** The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.
- -, /, I, Indicates required direction of slots; Reference "Joint Details" for more information.

20 Ga Gr40 connectors required

3X10-20HS - 20 Ga Gr60 connectors required

8X10-18HS - 18 Ga Gr60 connectors required

LATERAL BRACING

When this symbol shown, continuous lateral bracing is required on the web of the truss.



BEARING

Indicates location where bearings (supports) occur.



PLATE LOCATION & ORIENTATION

The plate shall be centered on joint and/or placed in accordance with the design drawing/QC full scale details.



REFERENCES

- •ANSI/TPI1: National Design Standard for Metal Plate Connected Wood Trusses
- •BCSI: Building Component & Safety
 Information Guide to Good Practice for
 Handling, Installing, Restraining, & Bracing of
 Metal Plate Connected Wood Trusses.
- •NDS: National Design Specification for Wood Construction
- •ESR: 1082 published by the International Code Council. www.icc-es.org

PAGE-19.1 1123362 0002/0018

Working Hard For Your Success 138 Wheeler Rd. / Clarkson, KY 42726 PH. (270) 242-9170 FX. 270-242-0217 Truss: G1

Rolling Acres (Display #3) Job:

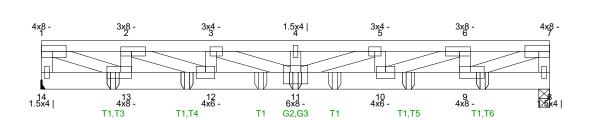
Designer: Shane Allen 03/31/20 11:36:04 Date:

Page: 1 of 2

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
13-9-0	0/12	2	0-0-0	0-0-0	0-0-0	0-0-0	2	24 in	77 lbs

2-3-8 2-3-8 2-3-8 2-3-8 2-3-8 2-3-8 2-3-8 4-7-0 6-10-8 9-2-0 11-5-8 13-9-0





0-0	0-0					0	0-0-0
	2-3-8	2-3-8	2-3-8	2-3-8	2-3-8	2-3-8	
	2-3-8	4-7-0	6-10-8	9-2-0	11-5-8	13-9-0	

All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection	ļ	L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2012/	TC:	0.71 (3-4)	Vert TL:	0.37 in	L/429	11	L/240
TCDL: 10		TPI 1-2007	BC:	0.63 (10-11)	Vert LL:	0.15 in	L/999	11	L/360
BCLL: 0	Rep Mbr:	No	Web:	0.80 (7-9)	Horz TL:	0.03 in		8	
BCDL: 10	Lumber D.O.L.	: 115 %							02/21/

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift
14	1	1.5 in	_	3,035 lbs		-670 lbs	•	-670 lbs
8	1	3.5 in	1.50 in	3,069 lbs		-678 lbs		-678 lbs

Material

TC: SYP#1 2 x 4 BC: SP2400/2.0 2 x 6 Web: SYP#2 2 x 4

Bracing

TC: Sheathed or Purlins at 3-6-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (10 psf) sloped roof snow loads in accordance with ASCE7 - 10 with the following user defined input: 10 psf GSL, Terrain C, Exposure (Ce = 1.0), Risk Category II (I = 1.00), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer

2) This truss has not been designed for the effects of unbalanced snow loads.

3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=25 ft, End Zone Truss, Bothend webs considered. DOL = 1.60

4) Minimum storage attic loading has been applied in accordance with IBC 1607.1

1-13	0.799	3,256 lbs	(-720 lbs)	3-11
2-13	0.113	-987 lbs		5-11
2-12	0.595	2,423 lbs	(-521 lbs)	5-10

Truss to Truss Connection Summary										
Carried Trus s	Carrying Chord	Carrying Offset								
T1	BC	1-11-4								
T3	BC	1-11-4								
T1	BC	3-11-4								
T4	BC	3-11-4								
T1	BC	5-11-4								
G2	BC	6-10-8								
G3	BC	6-10-8								
T1	BC	7-11-4								
T1	BC	9-11-4								
T5	BC	9-11-4								
T1	BC	11-11-4								
T6	BC	11-11-4								

Member Forces Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this -2,979 lbs 0.710 -6,296 lbs 0.532 -5,221 lbs **APPROVED** 0.532 -5.219 lbs 0.710 -6.296 lbs 0.306 -2.989 lbs 9-10 0.344 2.989 lbs 11-12 0.634 5.219 lbs (-1.141 lbs 0.634 0.343 DATE 4/1/20 0.062 0.286 1.165 lbs (-229 lbs 0.113 -983 lbs PRS CORPORATION 1,164 lbs 3,267 lbs 0.062 -537 lbs 0.157 -1.369 lbs Cottage Grove, WI

> **Rolling Acres Cabins** 166 John Logsdon Cemetery Rd. Munfordville, KY 42765

RAC-10

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGNAND AVAILABLE FROM EAGLE UPON REQUEST: DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

Precision Truss & Metal Sales Truss: G1 Working Hard For Your Success Rolling Acres (Display #3) Job: 138 Wheeler Rd. / Clarkson, KY 42726 Designer: Shane Allen 03/31/20 11:36:04 Date: PH. (270) 242-9170 FX. 270-242-0217 Page: 2 of 2SPAN PITCH OHL OHR CANTL CANTR PLYS **SPACING** WGT/PLY QTY 13-9-0 0/12 0-0-0 0-0-0 0-0-00-0-0 24 in 77 lbs

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Hanger is for graphical intrepretation only. Install hanger per manufacturer's recommendation.
- 3) Provide adequate drainage to prevent ponding.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) A creep factor of 1.50 has been applied for this truss analysis.
- 6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 7) The forces shown for this multi-ply truss are per ply and the reactions are for all plies. Two identical trusses shall be built and attached as follows, per ply: 10d Nails or Gun Nails[min .12"x2] TC-1 row @ 12 in oc, BC-2 staggered rows @ 12 in oc, Webs 1 row @ 12 in oc

Provided the hanger connections do not adequately transfer the applied load to all plies: in addition to connectors shown above, attach each pair of girder plies with supplemental 10d Nails or Gun Nails[min .12"x2] as follows within 24" of the location shown:

BC: 6-10-8,(14)Connectors

Connectors shall not encroach on other girder ply connectors or truss-to-truss connectors in accordance with the NDS or the connector manufacturer recommendations. 8) When applied loads are on one side of girder, do not flip girder during girder connector installation, install connectors on the girder side where supported loads are applied. When applied loads are on both sides of girder, double the spacing and install half of the connectors on one side of girder and then flip the girder to install the other half of the connectors on the opposite side (at double the connector spacing). Connectors on opposite sides of the girder shall be offset.

- 9) Lateral bracing shall be attached to each ply.
- 10) All fasteners minimum 2-1/2" long, unless otherwise noted.
- 11) Nails in 1st and 2nd ply shall be offset from successive plies by 1/2 the nail spacing .
- 12) Listed wind uplift reactions based on MWFRS & C&C loading.



Rolling Acres Cabins 166 John Logsdon Cemetery Rd. Munfordville, KY 42765

RAC-10

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Precision Truss & Metal

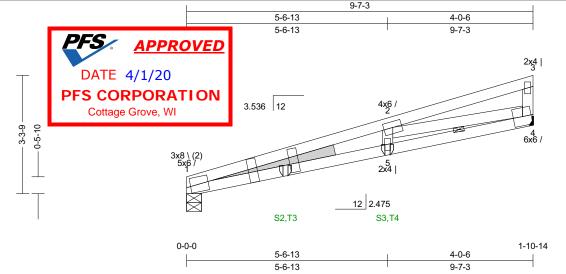
138 Wheeler Rd. Clarkson, KY 42726 Truss: G2

Rolling Acres (Display #3) Job:

Designer: Shane Allen 03/31/20 11:36:05 Date:

Page: 1 of 1

PITCH OHL CANTL CANTR PLYS **SPACING** WGT/PLY SPAN QTY OHR 9-7-3 3.536/12 0-0-0 0-0-0 0-0-0 0-0-0 24 in 42 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code:	IBC 2012/	TC:	0.66 (1-2)	Vert TL:	0.34 in	L/317	(5-1)	L/240
TCLL: 20	_	TPI 1-2007	BC:	0.90 (5-1)	Vert LL:	0.18 in UP	L/606	(5-1)	L/360
TCDL: 10	Rep Mbr:	No	Web:	0.54 (2-4)	Horz TL:	0.05 in		4	
BCLL: 0	Lumber D.O.L.:	115 %							
BCDL: 10									02/21

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	t Max C&C Uplift	Max Uplift	Max Horiz
1	1	4.907 in	1.50 in	1,151 lbs	•	-185 lbs	-524 lbs	-524 lbs	121 lbs
4	1	1.5 in		929 lbs	•	-178 lbs	-409 lbs	-409 lbs	

Material

TC: SP2400/2.0 2 x 4 BC: SP2400/2.0 2 x 4 Web: SYP#2.2 x 4

TC: Sheathed or Purlins at 3-5-0, Purlin design by Others. Sheathed or Purlins at 5-5-0, Purlin design by Others. Web: One Midpoint Row: 2-4

Loads

1) This truss has been designed for the effects of balanced (7 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 10 with the following user defined input: 10 psf GSL, Terrain C, Exposure (Ce = 1.0), Risk Category II (I = 1.00), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.

2) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, Not End Zone Truss, Both end webs considered. DOL=1.60

3) Minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.662	-4,204 lbs						
BC	4-5	0.432	4,134 lbs	(-2,037 lbs)	5-1	0.897	4,170 lbs	(-2,054 lbs)	
Web	2-5	0.183	746 lbs	(-322 lbs)	2-4	0.539	-4.031 lbs		

Truss to Truss Connection Summary

Carried Truss	Carrying Chord	Carrying Offset
S2	BC	2-8-14
T3	BC	2-8-14
S3	BC	5-6-13
T/1	RC.	5.6.13

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 25% (Cq = 0.75).
- 3) Hanger is for graphical intrepretation only. Install hanger per manufacturer's recommendation.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) A creep factor of 1.50 has been applied for this truss analysis.
- 6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.

 7) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
- 8) Indicates non-structural members.
- 9) Listed wind uplift reactions based on MWFRS & C&C loading

Rolling Acres Cabins 166 John Logsdon Cemetery Rd. Munfordville, KY 42765

RAC-10

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TrueBuild®Truss Software V5.6.355 Eagle Metal Products 23362 0005/0018

Precision Truss & Metal

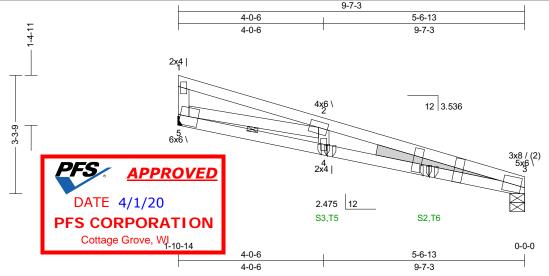
138 Wheeler Rd. Clarkson, KY 42726 Truss: G3

Rolling Acres (Display #3) Job:

Designer: Shane Allen 03/31/20 11:36:06 Date:

Page: 1 of 1

SPAN PITCH OHL CANTL CANTR **SPACING** WGT/PLY QTY OHR PLYS 9-7-3 -3.536/12 0-0-0 0-0-0 0-0-0 0-0-0 24 in 42 lbs



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code:	IBC 2012/	TC:	0.65 (2-3)	Vert TL:	0.35 in	L/313	(3-4)	L/240
TCLL: 20	-	TPI 1-2007	BC:	0.92 (3-4)	Vert LL:	0.18 in UP	L/604	(3-4)	L/360
TCDL: 10	Rep Mbr:	No	Web:	0.54 (2-5)	Horz TL:	0.05 in		3	
BCLL: 0	Lumber D.O.L.:	115 %							
BCDI · 10									

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
5	1	1.5 in		937 lbs	•	-155 lbs	-395 lbs	-395 lbs	-121 lbs
3	1	4.907 in	1.50 in	1,180 lbs	•	-216 lbs	-563 lbs	-563 lbs	

Material

TC: SP2400/2.0 2 x 4 BC: SP2400/2.0 2 x 4 Web: SYP#2.2 x 4

Bracing

TC: Sheathed or Purlins at 3-5-0, Purlin design by Others. Sheathed or Purlins at 5-6-0, Purlin design by Others.

Web: One Midpoint Row: 2-5

Loads

1) This truss has been designed for the effects of balanced (7 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 10 with the following user defined input: 10 psf GSL, Terrain C, Exposure (Ce = 1.0), Risk Category II (I = 1.00), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hit/gable Building Designer shall verify snow loads.

2) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=15 ft, Not End Zone Truss, Both end webs considered. DOL=1.60

3) Minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table. 4,215 lbs 0.918 4,226 lbs 4 145 lbs

Truss to Truss Connection Summary

Carried Truss	Carrying Chord	Carrying Offset
S3	BC	4-0-6
T5	BC	4-2-8
S2	BC	6-10-5
T6	BC	7-0-7

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) The fabrication tolerance for this roof truss is 25% (Cq = 0.75).
- 3) Hanger is for graphical intrepretation only. Install hanger per manufacturer's recommendation.
- 4) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 5) A creep factor of 1.50 has been applied for this truss analysis.
- 6) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.

 7) Indicates lateral bracing required perpendicular to the plane of the truss at either the midpoint (one shown) or third points (two shown), bracing by others. See BCSI-B3 for additional information.
- Indicates non-structural members.
- 9) Listed wind uplift reactions based on MWFRS & C&C loading.

Rolling Acres Cabins 166 John Logsdon Cemetery Rd. Munfordville, KY 42765

RAC-10

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGNAND AVAILABLE FROM EAGLE UPON REQUEST: DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

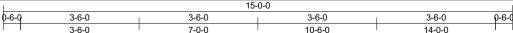
Working Hard For Your Success 138 Wheeler Rd. / Clarkson, KY 42726 PH. (270) 242-9170 FX. 270-242-0217 Truss: G4

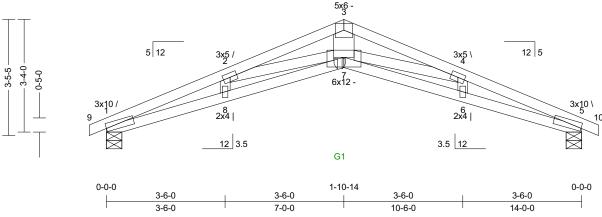
Rolling Acres (Display #3) Job:

Designer: Shane Allen 03/31/20 11:36:08 Date:

Page: 1 of 2

H	15-0-0											
	14-0-0	5/12	2	0-6-0	0-6-0	0-0-0	0-0-0	2	24 in	59 lbs		
l	SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY		





All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
Carried Loads (psf)	Bldg Code:	IBC 2012/	TC:	0.63 (3-4)	Vert TL:	0.54 in	L/290	7	L/240
TCLL: 20		TPI 1-2007	BC:	0.84 (5-6)	Vert LL:	0.21 in	L/730	(6-7)	L/360
TCDL: 10	Rep Mbr:	No	Web:	0.36 (4-7)	Horz TL:	0.39 in		5	
BCLL: 0	Lumber D.O.L.:	115 %							
BCDL: 10									03/31/

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	5.5 in	1.50 in	2,205 lbs	•	-500 lbs	-45 lbs	-500 lbs	•
5	1	5.5 in	1.50 in	2,160 lbs	•	-509 lbs	-54 lbs	-509 lbs	

Material

TC: SP2400/2.0 2 x 4 BC: SP2400/2.0 2 x 4 Web: SYP#2 2 x 4 except: SP 2400/2.0 2 x 8: 3-7

Bracing

TC: Sheathed or Purlins at 5-1-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

1) This truss has been designed for the effects of balanced (7 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 10 with the following user defined input: 10 psf GSL, Terrain C, Exposure (Ce = 1.0), Risk Category II (I = 1.00), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.

- 2) This truss has been designed to account for the effects of ice dams forming at the eaves.
- 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 10 with the following user defined input: 115 mph (Factored), Exposure C,

Enclosed, Gable/Hip, Risk Category II, h=B=L=25 ft, End Zone Truss, Bothend webs considered. DOL = 1.60

4) Minimum storage attic loading has been applied in accordance with IBC 1607.1

Load Case Lr1: Std Live Load

Distributed Loads

Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width
Тор	7-10-8	12-11-4	Down	Proj	20.63 plf	20.63 plf	
Top	-0-6-0	14-6-0	Down	Proj	19.37 plf	19.37 plf	
Top	-0-6-0	5-10-8	Down	Proj	20.63 plf	20.63 plf	



Load Case D1: Std Dead Load

Distributed Loads							- ··· · · · · · · · · · ·					
Member	Location 1	Location 2	Direction	Spread	Start Load	End Load	Trib Width Rolling Acres Cabins					
Тор	7-10-8	12-11-4	Down	Proj	10.31 plf	10.31 plf	166 John Lagadan Camatan, Dd					
Bot	7-10-8	12-11-4	Down	Proj	10.31 plf	10.31 plf	166 John Logsdon Cemetery Rd.					
Top	-0-6-0	14-6-0	Down	Proj	9.69 plf	9.69 plf	Munfordville, KY 42765					
Top	-0-6-0	5-10-8	Down	Proj	10.31 plf	10.31 plf	Wallordville, IXT 42703					
Bot	0-0-0	14-0-0	Down	Proj	9.69 plf	9.69 plf						
Bot	0-0-0	5-10-8	Down	Proj	10.31 plf	10.31 plf						
Bot	12-11-4	14-0-0	Down	Proj	10.31 plf	10.31 plf	RAC-10					
Mombon Fo	Mombon Former											

111(1)		roico	Idole	midicalcs. IVI	anoa	iD, likk CSi,	nux axia ioicc,	, (iikk compi. ioic	I dinacii no	TITTIKK ANIA ION	c). Only loice	s gicaici	ukui 500ios	ac snown in u	ns table.
TC	1-2	0.592	-4,762 lbs		3-4	0.635	-6,205 lbs					l			
	2-3	0.632	-6,205 lbs		4-5	0.592	-4,757 lbs								
BC	5-6	0.837	4,422 lbs	(-947 lbs)	6-7	0.806	4,509 lbs	(-966 lbs) 7-8	0.805	4,511 lbs	(-966 lbs)	8-1	0.835	4,424 lbs	(-947 lbs)

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Working Hald For Total Stacess 138 Wheeler Rd. / Clarkson, KY 42726 PH. (270) 242-9170 FX. 270-242-0217 Truss: G4

Job: Rolling Acres (Display #3)

Designer: Shane Allen Date: 03/31/20 11:36:08

Page: 2 of 2

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
14-0-0	5/12	2	0-6-0	0-6-0	0-0-0	0-0-0	2	24 in	59 lbs

Web 2-7 0.355 1,447 lbs (-280 lbs) 4-7 0.356 1,449 lbs (-280 lb 3-7 0.188 4,529 lbs (-945 lbs)

Truss to Truss Connection Summary Carried Truss Carrying Chord Carrying Offset

Note

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 3) A creep factor of 1.50 has been applied for this truss analysis.
- 4) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 5) The forces shown for this multi-ply truss are per ply and the reactions are for all plies. Two identical trusses shall be built and attached as follows, per ply. 10d Nails or Gun Nails[min .12'x2] TC-1 row @ 12 in oc, BC-1 row @ 12 in oc, Webs less than 2x8-1 row @ 12 in oc, Webs 2x8-2 rows @ 12 in oc, staggered.

Provided the hanger connections do not adequately transfer the applied load to all plies: in addition to connectors shown above, attach each pair of girder plies with supplemental 10d Nails or Gun Nails[min .12"x2] as follows within 24" of the location shown:

BC: 6-10-8,(22)Connectors

Connectors shall not encroach on other girder ply connectors or truss-to-truss connectors in accordance with the NDS or the connector manufacturer recommendations.

6) When applied loads are on one side of girder, do not flip girder during girder connector installation, install connectors on the girder side where supported loads are applied. When applied loads are on both sides of girder, double the spacing and install half of the connectors on one side of girder and then flip the girder to install the other half of the connectors on the opposite side (at double the connector spacing). Connectors on opposite sides of the girder shall be offset.

- 7) Lateral bracing shall be attached to each ply.
- 8) All fasteners minimum 2-1/2" long, unless otherwise noted.
- 9) Nails in 1st and 2nd ply shall be offset from successive plies by 1/2 the nail spacing.
- 10) Listed wind uplift reactions based on MWFRS & C&C loading.



Rolling Acres Cabins 166 John Logsdon Cemetery Rd. Munfordville, KY 42765

RAC-10

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Working Hard For Your Success 138 Wheeler Rd. / Clarkson, KY 42726 PH. (270) 242-9170 FX. 270-242-0217 Truss: GE1

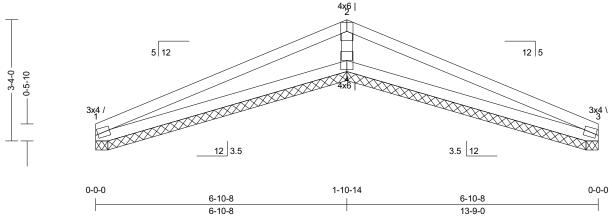
Job: Rolling Acres (Display #3)

Designer: Shane Allen Date: 03/31/20 11:36:09

Page: 1 of 1

SPAN PITCH QTY OHL OHR CANTL CANTR PLYS SPACING WGT/PLY 13-9-0 5/12 0-0-0 0-0-0 0-0-0 0-0-0 24 in 43 lbs

> 13-9-0 6-10-8 6-10-8 13-9-0



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2012/	TC:	0.50 (1-2)	Vert TL:	$0.04 \mathrm{in}$	L/999	(3-4)	L/240
TCDL: 10	•	TPI 1-2007	BC:	0.17 (4-1)	Vert LL:	0 in UP	L/999	(4-1)	L/360
BCLL: 0	Rep Mbr:	No	Web:	0.06 (2-4)	Horz TL:	0 in			
BCDL: 10	Lumber D.O.L.	: 115 %							

Reaction

Brg Combo Brg Width Max React Ave React Max Grav Uplift Max MWFRS Uplift Max C&C Uplift Max Uplift Max Uplift Max Uplift Max Horiz

1 1,351 lbs 344 plf -1,018 lbs -439 lbs -1,018 lbs -1,018 lbs 680 lbs

Material

TC: SYP#1 2 x 4 BC: SYP#1 2 x 4 Web: SYP#2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (7 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 10 with the following user defined input: 10 psf GSL, Terrain C, Exposure (Ce = 1.0), Risk Category II (I = 1.00), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip gable Building Designer shall verify snow loads.

2) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=25 ft, End Zone Truss, Bothend webs considered. DOL=1.60

Men	nber i	Forces	Table	e indicates: Memb	ber ID, max CSI,	max axial force,	, (max compr.	force if different from max axial force). Only force	greater than 300lbs are shown in this table.
TC	1-2	0.500	1,096 lbs	(-849 lbs) 2-	3 0.500	1,096 lbs	(-849 lbs)		
BC									
Weh	24	0.058	332 lbs	(-307 lbs)					

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Gable requires continuous bottom chord bearing.
- 3) Gable webs placed at 24 "OC, U.N.O.
- 4) Attach gable webs with 3x4 20ga plates, U.N.O.
- 5) Bracing shown is for in-plane requirements. For out-of-plane requirements, refer to BCSI-B3 published by the SBCA.
- 6) A creep factor of 1.50 has been applied for this truss analysis.
- 7) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 8) Due to negative reactions in gravity load cases, special connections to the bearing surface at joints 3, 1 may need to be considered.
- 9) Listed wind uplift reactions based on MWFRS & C&C loading.



Rolling Acres Cabins 166 John Logsdon Cemetery Rd. Munfordville, KY 42765

RAC-10

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Working Hard For Your Success 138 Wheeler Rd. / Clarkson, KY 42726 PH. (270) 242-9170 FX. 270-242-0217 Truss: S1

Job: Rolling Acres (Display #3)

WCT/DLV

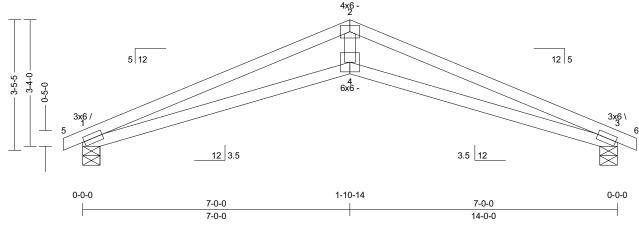
Designer: Shane Allen
Date: 03/31/20 11:36:10

Page: 1 of 1

	i			15.	-0-0			1		_
14-0-0	5/12	11	0-6-0	0-6-0	0-0-0	0-0-0	1	24 in	46 lbs	
SPAIN	PHCH	QII	OHL	OHK	CANIL	CANTR	PLIS	SPACING	WGI/PLY	

CANTED

15-0-0 0-6-0 7-0-0 7-0-0 0-6-C 7-0-0 14-0-0



All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2012/	TC:	0.53 (1-2)	Vert TL:	0.32 in	L/489	(3-4)	L/240
TCDL: 10		TPI 1-2007	BC:	0.62 (4-1)	Vert LL:	0.13 in UP	L/999	4	L/360
BCLL: 0	Rep Mbr:	Yes	Web:	0.39 (2-4)	Horz TL:	0.24 in		3	
RCDI · 10	Lumber DOL	115 %							

Reaction

CDANI

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplif	t Max C&C Uplift	Max Uplift	Max Horiz
1	1	5.5 in	1.50 in	730 lbs	•	-152 lbs	-397 lbs	-397 lbs	4 lbs
3	1	5.5 in	1.50 in	730 lbs	•	-152 lbs	-397 lbs	-397 lbs	•
					_	_			

Material

TC: SYP#1 2 x 4
BC: SYP#1 2 x 4
Web: SYP#2 2 x 4

Bracing

TC: Sheathed or Purlins at 3-5-0, Purlin design by Others. BC: Sheathed or Purlins at 6-7-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (7 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 10 with the following user defined input: 10 psf GSL, Terrain C, Exposure (Ce = 1.0), Risk Category II (I = 1.00), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.

2) This truss has been designed to account for the effects of ice dams forming at the eaves.

3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 115 mph (Factored), Exposure C,

Enclosed, Gable/Hip, Risk Category II, h=B=L=25 ft, End Zone Truss, Bothend webs considered. DOL=1.60

OTM

OTI

4) Minimum storage attic loading has been applied in accordance with IBC 1607.1 $\,$

Men	nber	Forces	Table	indicates: M	ember I	D, max CSI,	max axial force,	, (max compr.	force if different from max axial force). Only forces	greater than 300lbs are shown in this
TC	1-2	0.531	-2,603 lbs		2-3	0.531	-2,603 lbs			
BC	3-4	0.621	2,454 lbs	(-953 lbs)	4-1	0.621	2,454 lbs	(-953 lbs)		
Web	2-4	0.387	1,577 lbs	(456 lbs)						

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 3) A creep factor of 1.50 has been applied for this truss analysis.
- 4) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 5) Listed wind uplift reactions based on MWFRS & C&C loading.



Rolling Acres Cabins 166 John Logsdon Cemetery Rd. Munfordville, KY 42765

RAC-10

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

Working Hard For Your Success 138 Wheeler Rd. / Clarkson, KY 42726 PH. (270) 242-9170 FX. 270-242-0217 Truss: S2

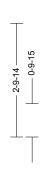
Rolling Acres (Display #3) Job:

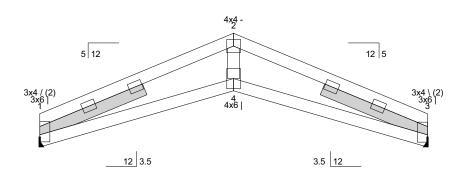
Designer: Shane Allen 03/31/20 11:36:11 Date:

Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
9-6-14	5/12	2	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	40 lbs

4-9-7 4-9-7 4-9-7 9-6-14





0-0-0	1	-4-12	0-0-0
	4-9-7	4-9-7	
	4-9-7	9-6-14	

All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2012/	TC:	0.24 (2-3)	Vert TL:	$0.06 \mathrm{in}$	L/999	(3-4)	L/240
TCDL: 10		TPI 1-2007	BC:	0.31 (4-1)	Vert LL:	$0.02 \mathrm{in}$	L/999	4	L/360
BCLL: 0	Rep Mbr:	No	Web:	0.18 (2-4)	Horz TL:	0.05 in		3	
BCDL: 10	Lumber D.O.L.	: 115 %							

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	t Max C&C Uplift	Max Uplift	Max Horiz
1	1	1.5 in	_	479 lbs	•	-93 lbs	-274 lbs	-274 lbs	
3	1	1.5 in		479 lbs	•	-93 lbs	-274 lbs	-274 lbs	

Material

TC: SYP#1 2 x 4 BC: SYP#1 2 x 4 Web: SYP#2 2 x 4

Bracing

TC: Sheathed or Purlins at 5-8-0, Purlin design by Others. BC: Sheathed or Purlins at 9-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (7 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 10 with the following user defined input: 10 psf GSL, Terrain C, Exposure (Ce = 1.0), Risk Category II (I = 1.00), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.

2) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=25 ft, End Zone Truss, Bothend webs considered. DOL = 1.60

3) Minimum storage attic loading has been applied in accordance with IBC 1607.1

Member Forces Table indicates: Member ID, max CSI, max axial force, (max compr. force if different from max axial force). Only forces greater than 300lbs are shown in this table

TC	1-2	0.238	-1,144 lbs		2-3	0.238	-1,144 lbs		
BC	3-4	0.309	1,088 lbs	(-468 lbs)	4-1	0.309	1,088 lbs	(468 lbs)	
Web	2-4	0.180	733 lbs	(-227 lbs)					

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Hangers are for graphical intrepretation only. Install hangers per manufacturer's recommendations.
- 3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 4) A creep factor of 1.50 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
 6) Indicates non-structural members.
- 7) Listed wind uplift reactions based on MWFRS & C&C loading.



Rolling Acres Cabins 166 John Logsdon Cemetery Rd. Munfordville, KY 42765

RAC-10

ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCTS DESIGN NOTES ISSUED WITH THIS DESIGNAND AVAILABLE FROM EAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

TrueBuild®Truss Software v5.6.355 Eagle Metal Products

23362 0011/0018

Working Hard For Your Success 138 Wheeler Rd. / Clarkson, KY 42726 PH. (270) 242-9170 FX. 270-242-0217 Truss: S3

Rolling Acres (Display #3) Job:

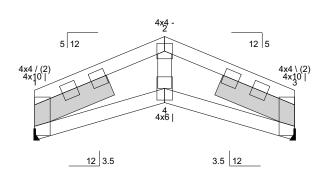
Designer: Shane Allen 03/31/20 11:36:12 Date:

Page: 1 of 1

SPAN PITCH OHL OHR CANTL CANTR PLYS **SPACING** WGT/PLY QTY 5-6-14 5/12 0-0-0 0-0-0 0-0-00-0-0 24 in $30 \, \mathrm{lbs}$

5-6	-14
2-9-7	2-9-7
2-9-7	5-6-14





0-0-0		0-9-12		0-0-0
1	2-9-7	1	2-9-7	1
	2-9-7		5-6-14	

All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2012/	TC:	0.09 (2-3)	Vert TL:	0.01 in	L/999	(3-4)	L/240
TCDL: 10		TPI 1-2007	BC:	0.13 (4-1)	Vert LL:	0 in	L/999	4	L/360
BCLL: 0	Rep Mbr:	No	Web:	0.06 (2-4)	Horz TL:	0.01 in		3	
BCDL: 10	Lumber D.O.L.:	115 %							

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	
1	1	1.5 in		279 lbs	•	-54 lbs	-185 lbs	-185 lbs	
3	1	1.5 in		279 lbs	•	-54 lbs	-185 lbs	-185 lbs	

Material

TC: SYP#1 2 x 4 BC: SYP#1 2 x 4 Web: SYP#2.2 x 4

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (10 psf) sloped roof snow loads in accordance with ASCE7 - 10 with the following user defined input/10 p. GSL, Terrain C, Exposure (Ce = 1.0), Risk Category II (I = 1.00), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip gable, Building Designer

- 2) This truss has not been designed for the effects of unbalanced snow loads.
- 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 10 with the following user defined input: 115 mph (Factored), Exposure C,

Enclosed, Gable/Hip, Risk Category II, h=B=L=25 ft, End Zone Truss, Bothend webs considered. DOL = 1.60

4) Minimum storage attic loading has been applied in accordance with IBC 1607.1

]	Mem	ber 1	Forces	Table	indicates: M	ember]	ID, max CSI, 1	max axial force,	(max compr.	force if different from max axial force). Only force	es greater than 300lbs are shown in this table
	ГC	1-2	0.091	-372 lbs		2-3	0.091	-372 lbs			
Ī	3C	3-4	0.131	338 lbs	(-119 lbs)	4-1	0.131	338 lbs	(-119 lbs)		
7	Web										

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Hangers are for graphical intrepretation only. Install hangers per manufacturer's recommendations.
- 3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 4) A creep factor of 1.50 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
 6) Indicates non-structural members.
- 7) Listed wind uplift reactions based on MWFRS & C&C loading.



Rolling Acres Cabins 166 John Logsdon Cemetery Rd. Munfordville, KY 42765

RAC-10

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Working Hard For Your Success 138 Wheeler Rd. / Clarkson, KY 42726 PH. (270) 242-9170 FX. 270-242-0217 Truss: T1

Job: Rolling Acres (Display #3)

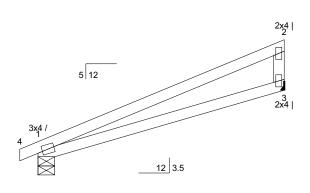
Designer: Shane Allen 03/31/20 11:36:13 Date:

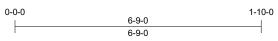
Page: 1 of 1

SPAN PITCH QTY OHL OHR CANTL CANTR PLYS **SPACING** WGT/PLY 6-9-0 5/12 12 0-6-0 0-0-0 0-0-0 0-0-0 24 in 22 lbs

> 7-3-0 6-9-0 6-9-0







All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2012/	TC:	0.59 (1-2)	Vert TL:	0.25 in	L/298	(3-1)	L/240
TCDL: 10		TPI 1-2007	BC:	0.51 (3-1)	Vert LL:	0.13 in UP	L/558	(3-1)	L/360
BCLL: 0	Rep Mbr:	Yes	Web:	0.06 (2-3)	Horz TL:	0 in		3	
BCDL: 10	Lumber D.O.L.	: 115 %							02/21/

Reaction

JI	Dig Combo	Dig widin	riqu big widin	Max React	Max Grav Opint	Max MWI NO OPHIL	Max C&C Opin	Max Opini
1	1	5.5 in	1.50 in	361 lbs	•	-50 lbs	-216 lbs	-216 lbs
3	1	1.5 in	_	344 lbs	•	-97 lbs	-235 lbs	-235 lbs

Material

TC: SYP#1 2 x 4 BC: SYP#1 2 x 4 Web: SYP#2.2 x 4

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others.

BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (7 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 10 with the following user defined input: 10 psf GSL, Terrain C, Exposure (Ce = 1.0), Risk Category II (I = 1.00), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.

2) This truss has been designed to account for the effects of ice dams forming at the eaves.

3) This truss has been designed for the effects of wind loads in accordance with ASCE7 - 10 with the following user defined input: 115 mph (Factored), Exposure C,

Enclosed, Gable/Hip, Risk Category II, h=B=L=25 ft, End Zone Truss, Bothend webs considered. DOL=1.60

4) Minimum storage attic loading has been applied in accordance with IBC 1607.1

Men	nber l	Forces	Table indicates: M	ember ID, max CSI, max axial force, (max compr.	force if different from max axial force). Only forces	greater than 300lbs are shown in this
TC	1-2	0.590	-402 lbs		•	
BC						
Web						

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Hanger is for graphical intrepretation only. Install hanger per manufacturer's recommendation.
- 3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 4) A creep factor of 1.50 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
- 6) Listed wind uplift reactions based on MWFRS & C&C loading.



Rolling Acres Cabins 166 John Logsdon Cemetery Rd. Munfordville, KY 42765

RAC-10

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Working Hard For Your Success 138 Wheeler Rd. / Clarkson, KY 42726 PH. (270) 242-9170 FX. 270-242-0217 Truss: T3

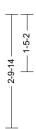
Rolling Acres (Display #3) Job:

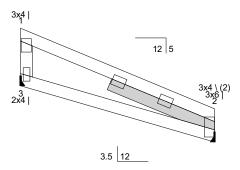
Designer: Shane Allen 03/31/20 11:36:14 Date:

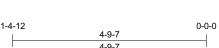
Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
4-9-7	-5/12	2	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	21 lbs

4-9-7 4-9-7 4-9-7







All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2012/	TC:	0.25 (1-2)	Vert TL:	0.02 in	L/999	(2-3)	L/240
TCDL: 10	_	TPI 1-2007	BC:	0.23 (2-3)	Vert LL:	0.01 in UP	L/999	(2-3)	L/360
BCLL: 0	Rep Mbr:	No	Web:	0.34 (1-3)	Horz TL:	0 in		2	
BCDL: 10	Lumber D.O.L.:	115 %							

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift
3	1	1.5 in		247 lbs	•	-46 lbs	-146 lbs	-146 lbs
2	1	1.5 in		232 lbs		-48 lbs	-163 lbs	-163 lbs

Material

TC: SYP#1 2 x 4 BC: SYP#1 2 x 4 Web: SYP#2.2 x 4

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (7 psf) sloped roof snow loads in accordance with ASCE7 - 10 with the following user defined input: 10 ps GSL, Terrain C, Exposure (Ce = 1.0), Risk Category II (I = 1.00), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer

- 2) This truss has not been designed for the effects of unbalanced snow loads.
- 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 10 with the following user defined input: 115 mph (Factored), Exposure C,

Enclosed, Gable/Hip, Risk Category II, h=B=L=25 ft, End Zone Truss, Bothend webs considered. DOL = 1.60

4) Minimum storage attic loading has been applied in accordance with IBC 1607.1

Men	nber 1	Forces	Table indicates: Ma	ember ID, max CSI, max axial force, (max compr.	force if different from max axial force). Only forces	greater than 300lbs are shown in this
TC	1-2	0.252	-312 lbs			
BC						
Web						

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Hangers are for graphical intrepretation only. Install hangers per manufacturer's recommendations.
- 3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 4) A creep factor of 1.50 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
 6) Indicates non-structural members.
- 7) Listed wind uplift reactions based on MWFRS & C&C loading.



Rolling Acres Cabins 166 John Logsdon Cemetery Rd. Munfordville, KY 42765

RAC-10

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Working Hard For Your Success 138 Wheeler Rd. / Clarkson, KY 42726 PH. (270) 242-9170 FX. 270-242-0217 Truss: T4

Job: Rolling Acres (Display #3)

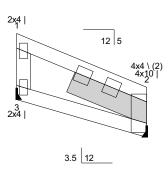
Designer: Shane Allen 03/31/20 11:36:15 Date:

Page: 1 of 1

SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
2-9-7	-5/12	2	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	16 lbs

2-9-7
2-9-7
2-9-7







All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2012/	TC:	0.10 (1-2)	Vert TL:	0 in	L/999	(2-3)	L/240
TCDL: 10	_	TPI 1-2007	BC:	0.07 (2-3)	Vert LL:	0 in UP	L/999	(2-3)	L/360
BCLL: 0	Rep Mbr:	No	Web:	0.14 (1-3)	Horz TL:	0 in		2	
BCDL: 10	Lumber D.O.L.:	: 115 %							

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
3	1	1.5 in		147 lbs	•	-35 lbs	-92 lbs	-92 lbs	-99 lbs
2	1	1.5 in		132 lbs	•	-20 lbs	-88 lbs	-88 lbs	•

Material

TC: SYP#1 2 x 4 BC: SYP#1 2 x 4 Web: SYP#2 2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (7 psf) sloped roof snow loads in accordance with ASCE7 - 10 with the following user defined input: 10 psf GSL, Terrain C, Exposure (Ce = 1.0), Risk Category II (I = 1.00), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer

- 2) This truss has not been designed for the effects of unbalanced snow loads.
- 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 10 with the following user defined input: 115 mph (Factored), Exposure C,

Enclosed, Gable/Hip, Risk Category II, h=B=L=25 ft, End Zone Truss, Bothend webs considered. DOL = 1.60

4) Minimum storage attic loading has been applied in accordance with IBC 1607.1

Mem	ber Forces	Table indicates: Me	ember ID, max CSI, max axial force, (max compr.	force if different from max axial force). Only for	orces greater than 300lbs are shown in this table.
TC					Ī
BC					

Web **Notes**

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Hangers are for graphical intrepretation only. Install hangers per manufacturer's recommendations.
- 3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 4) A creep factor of 1.50 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
 6) Indicates non-structural members.
- 7) Listed wind uplift reactions based on MWFRS & C&C loading.



Rolling Acres Cabins 166 John Logsdon Cemetery Rd. Munfordville, KY 42765

RAC-10

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Working Hard For Your Success 138 Wheeler Rd. / Clarkson, KY 42726 PH. (270) 242-9170 FX. 270-242-0217 Truss: T5

Rolling Acres (Display #3) Job:

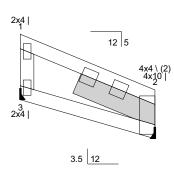
Designer: Shane Allen 03/31/20 11:36:16 Date:

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SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
2-10-15	-5/12	2	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	16 lbs

2-10-15
2-10-15
2-10-15







All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2012/	TC:	0.10 (1-2)	Vert TL:	0 in	L/999	(2-3)	L/240
TCDL: 10		TPI 1-2007	BC:	0.08 (2-3)	Vert LL:	0 in UP	L/999	(2-3)	L/360
BCLL: 0	Rep Mbr:	No	Web:	0.14 (1-3)	Horz TL:	0 in		2	
BCDL: 10	Lumber D.O.L.	115 %							

Reaction

JI	Dig Combo	Dig widii	rqu big widii	IVIAX INCACT	Max Grav Opint	Max MWT No Opint	Max Coc Opini	Max Opin
3	1	1.5 in	_	153 lbs	•	-35 lbs	-95 lbs	-95 lbs
2	1	1.5 in		138 lbs	•	-21 lbs	-93 lbs	-93 lbs

Material

TC: SYP#1 2 x 4 BC: SYP#1 2 x 4 Web: SYP#2.2 x 4

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

1) This truss has been designed for the effects of balanced (7 psf) sloped roof snow loads in accordance with ASCE7 - 10 with the following user defined input: 10 psf GSL, Terrain C, Exposure (Ce = 1.0), Risk Category II (I = 1.00), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Ilesigner

- 2) This truss has not been designed for the effects of unbalanced snow loads.
- 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 10 with the following user defined input: 115 mph (Factored), Exposure C,

Enclosed, Gable/Hip, Risk Category II, h=B=L=25 ft, End Zone Truss, Bothend webs considered. DOL = 1.60

4) Minimum storage attic loading has been applied in accordance with IBC 1607.1

Mem	iber Forces	Table indicates: Ma	ember ID, max CSI, max axial force, (max compr.	force if different from max axial force). Only forces	greater than 300lbs are shown in this
TC					
BC					
Web					

Notes

- 1) Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- 2) Hangers are for graphical intrepretation only. Install hangers per manufacturer's recommendations.
- 3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 4) A creep factor of 1.50 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
 6) Indicates non-structural members.
- 7) Listed wind uplift reactions based on MWFRS & C&C loading.



Rolling Acres Cabins 166 John Logsdon Cemetery Rd. Munfordville, KY 42765

RAC-10

Max Horiz

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TrueBuild®Truss Software v5.6.355 Eagle Metal Products

23362 0016/0019

Working Hard For Your Success 138 Wheeler Rd. / Clarkson, KY 42726 PH. (270) 242-9170 FX. 270-242-0217 Truss: T6

Rolling Acres (Display #3) Job:

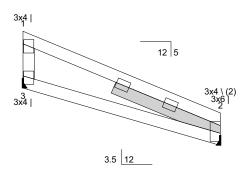
Designer: Shane Allen 03/31/20 11:36:17 Date:

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SPAN	PITCH	QTY	OHL	OHR	CANTL	CANTR	PLYS	SPACING	WGT/PLY
4-10-15	-5/12	2	0-0-0	0-0-0	0-0-0	0-0-0	1	24 in	21 lbs

4-10-15 4-10-15 4-10-15







1-5-3		0-0-0
i i	4-10-15	
	4-10-15	1

All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf)	General		CSI		Deflection		L/	(loc)	Allowed
TCLL: 20	Bldg Code:	IBC 2012/	TC:	0.26 (1-2)	Vert TL:	0.02 in	L/999	(2-3)	L/240
TCDL: 10		TPI 1-2007	BC:	0.24 (2-3)	Vert LL:	0.02 in UP	L/999	(2-3)	L/360
BCLL: 0	Rep Mbr:	No	Web:	0.36 (1-3)	Horz TL:	0 in		2	
BCDL: 10	Lumber D.O.L.:	115 %							

Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift
3	1	1.5 in		253 lbs		-47 lbs	-149 lbs	-149 lbs
2	1	1.5 in		238 lbs		-49 lbs	-168 lbs	-168 lbs

Material

TC: SYP#1 2 x 4 BC: SYP#1 2 x 4 Web: SYP#2.2 x 4

Bracing

TC: Sheathed or Purlins at 6-3-0, Purlin design by Others. BC: Sheathed or Purlins at 10-0-0, Purlin design by Others.

Loads

- 1) This truss has been designed for the effects of balanced (7 psf) sloped roof snow loads in accordance with ASCE7 10 with the following user defined input to the GSL, Terrain C, Exposure (Ce = 1.0), Risk Category II (I = 1.00), Thermal (Ct = 1.00), DOL = 1.15. If the roof configuration differs from hip/gable, Building Designer
- 2) This truss has not been designed for the effects of unbalanced snow loads.
- 3) This truss has been designed for the effects of wind loads in accordance with ASCE7 10 with the following user defined input: 115 mph (Factored), Exposure C, Enclosed, Gable/Hip, Risk Category II, h=B=L=25 ft, End Zone Truss, Bothend webs considered. DOL = 1.60
- 4) Minimum storage attic loading has been applied in accordance with IBC 1607.1

Men	nber 1	Forces	Table indicates:	Member ID, max CSI, max axial force, (max compr.	force if different from max axial force). Only forces	s greater than 300lbs are shown in this table.
TC	1-2	0.261	-339 lbs			
BC						
Web						

Notes

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- 3) Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- 4) A creep factor of 1.50 has been applied for this truss analysis.
- 5) The "SYP" label shown in the "Material Summary" above indicates the new SPIB design values effective June 1, 2013 were used.
 6) Indicates non-structural members.
- 7) Listed wind uplift reactions based on MWFRS & C&C loading.



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